



THE GERMAN EMPIRICAL – MECHANISTIC DESIGN SYSTEM

PART I – M - E ASPHALT PAVEMENT DESIGN IN GERMANY

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TU Dresden

NADim Seminar - Oslo 03.12.2015

Forschungsgesellschaft für Straßen und Verkehrswesen

Arbeitsgruppe Infrastrukturmanagement



Richtlinien
für die rechnerische Dimensionierung
des Oberbaus von Verkehrsflächen
mit Asphaltdeckschicht

R 1

RDO Asphalt

**Mechanistic approach
for pavement design**

Ausgabe 2009

Forschungsgesellschaft für Straßen und Verkehrswesen

Arbeitsgruppe Infrastrukturmanagement



Richtlinien
für die Standardisierung des
Oberbaus von Verkehrsflächen

R 1

RStO 12

Standardization

Ausgabe 2012



Surface layer

Binder course

Frost protection:

Entire thickness of pavement is dependent on:

- frost penetration
- frost susceptibility of the soil in combination with
- traffic load
- between 45 ... 90 cm



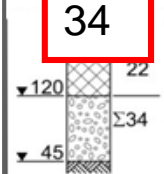
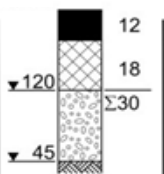
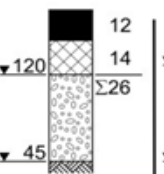
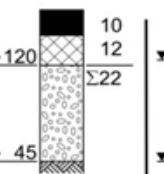
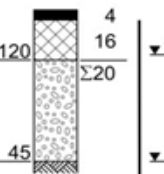
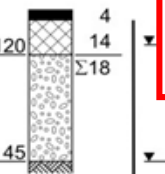
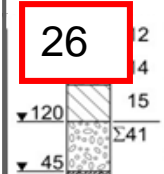
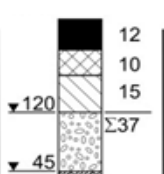
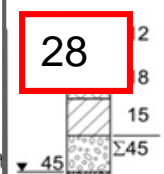
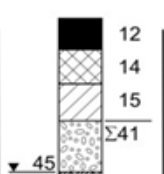
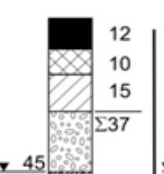
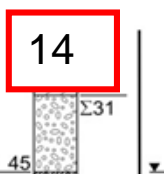
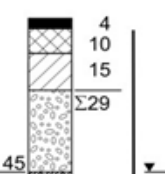
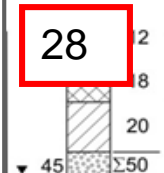
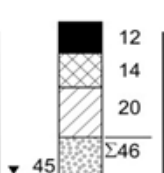
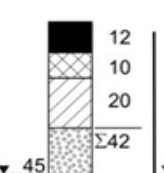
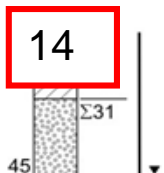
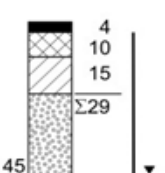
Surface layer

Binder layer

Base layer



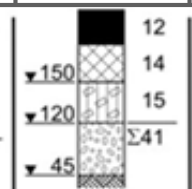
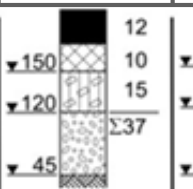
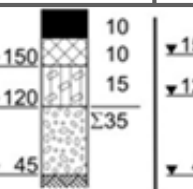
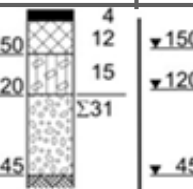
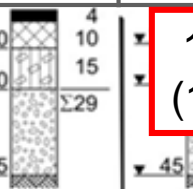
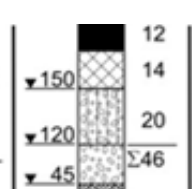
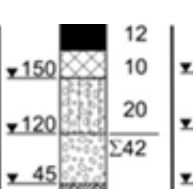
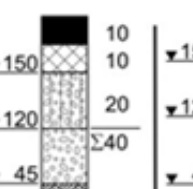
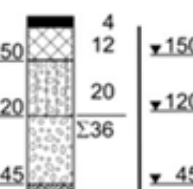
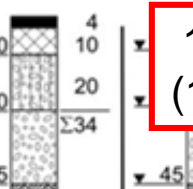
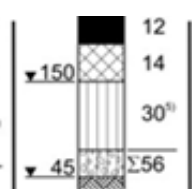
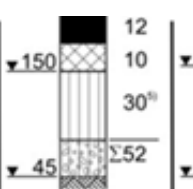
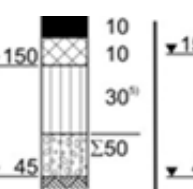
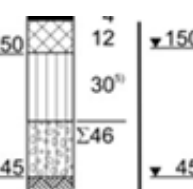
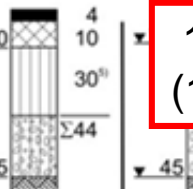
Base layer

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10t EASLs - B [Mio]	>32	32-10	10 – 3,2	3,3 – 1.8	1.8 – 1,0	1,0 – 0.3	<0,3
1 Asphalt FPL	55 65 75 85	55 65 75 85	55 65 75 85	45 55 65 75	45 55 65 75	35 45 55 65	35 45 55 65
	34						
2.1 Asphalt CTB FPL	31 ² 41 51	25 ³ 35 45 55	29 ³ 39 49 59	- 33 ² 43 53	25 ³ 35 45 55	17 ² 27 37 47	21 31 41 51
	26			20			
2.2 Asphalt CTB FPL	31 ² 41 51	25 ³ 35 45 55	29 ³ 39 49 59	- 33 ² 43 53	25 ³ 35 45 55	17 ² 27 37 47	21 31 41 51
	28				14		
2.3 Asphalt CTB FPL	31 ² 41 51	25 ³ 35 45 55	29 ³ 39 49 59	- 33 ² 43 53	25 ³ 35 45 55	17 ² 27 37 47	21 31 41 51
	28				14		
	5 ¹⁾ 15 ⁴ 25 35	9 ¹⁾ 19 ⁴ 29 39	13 ¹⁾ 23 33 43	5 ¹⁾ 15 ⁴ 25 35	14 ⁴ 24 34 44	6 ⁴⁾ 16 ⁴ 26 36	6 ⁴⁾ 16 ⁴ 26 36

RStO Table I, Part II



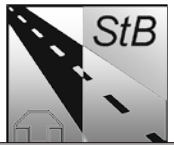
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LCC 10t EASLs - B [Mio]	100 >32	32 32-10	10 10 - 3.2	3.2 3.2 - 1.8	1.8 1.8 - 1.0	1.0 1.0 - 0.3	0.3 <0,3
3 Asphalt CRB FPL	30						14 (10)
	- - 30 ² 40	- - 34 ² 44	- 28 ³ 38 48	- - 30 ² 40	- 24 ³ 34 44	- 26 ³ 36 46	- 18 ³ 28 38
4 Asphalt GBC FPL	30						14 (10)
	- - 25 ³ 35	- - 29 ³ 39	- 33 ² 43	- - 25 ³ 35	- - 29 ² 39	- - 31 ² 41	- - 23 ³ 33
5 Asphalt CRB/GBC FPL	30						14 (10)
	- - 30 ² 40	- - 34 ² 44	- 28 ³ 38 48	- - 30 ² 40	- 24 ³ 34 44	- 26 ³ 36 46	- 18 ³ 28 38

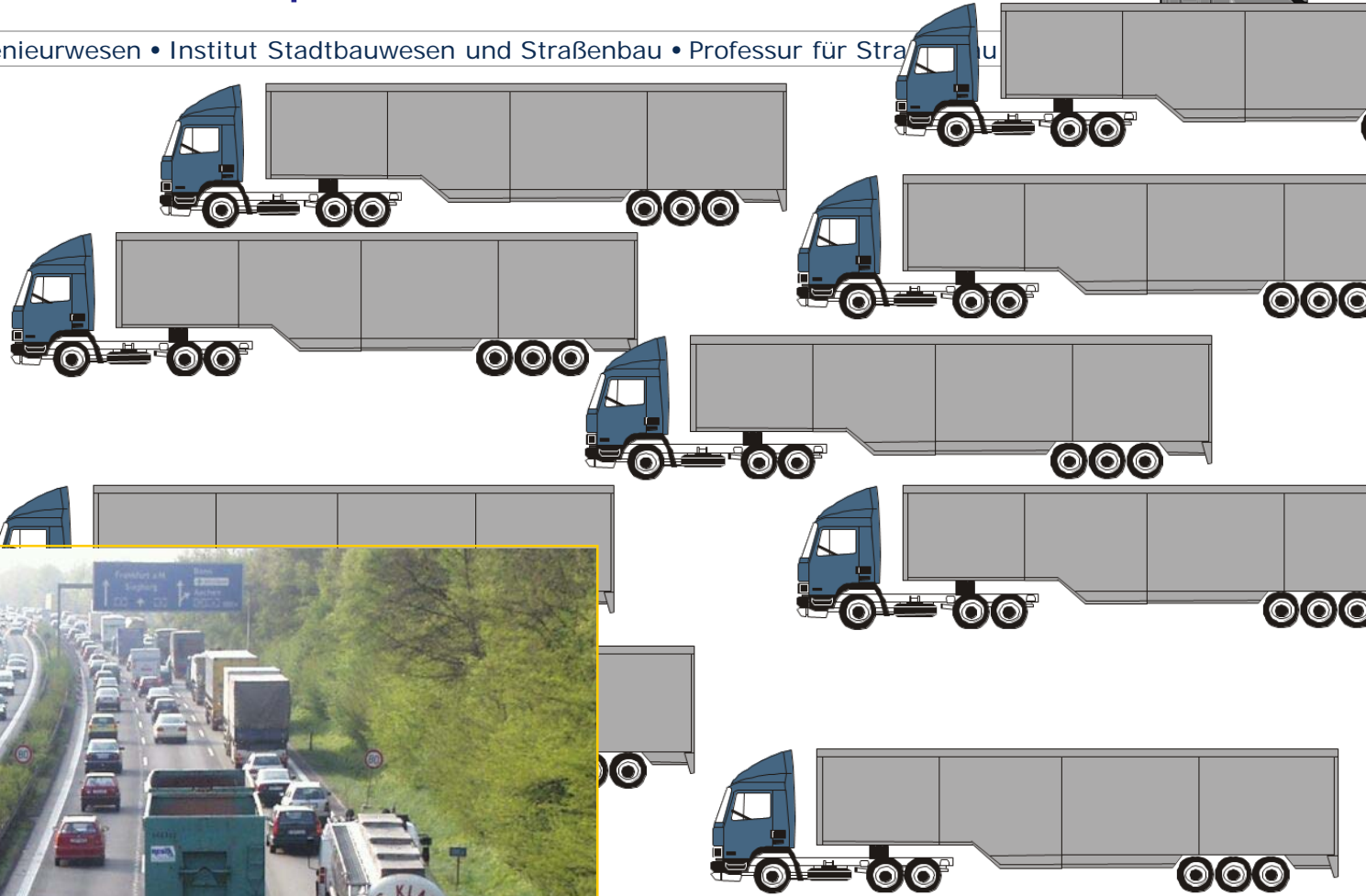
Why

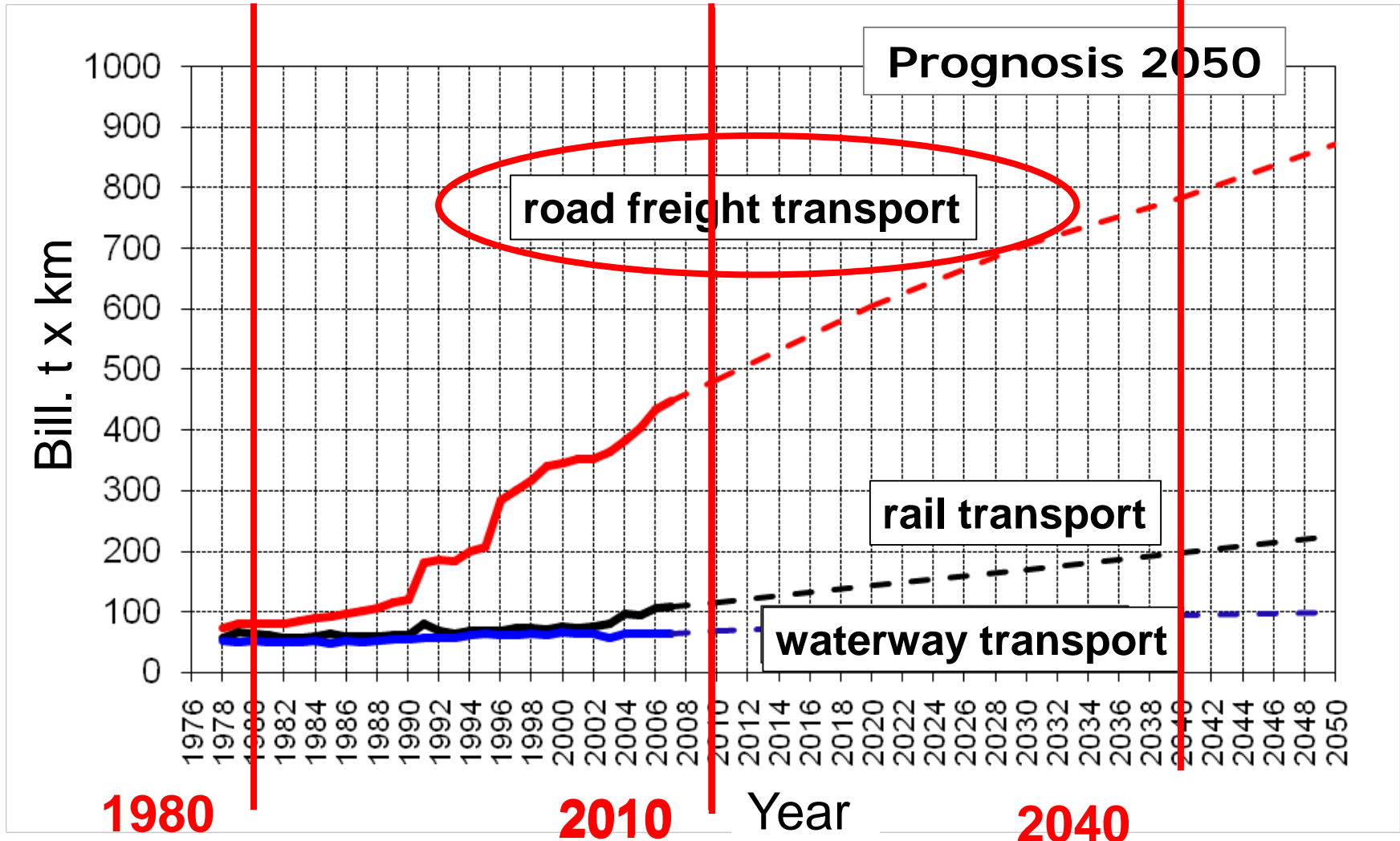
**a new mechanistic approach
for pavement design ?**

Development of traffic



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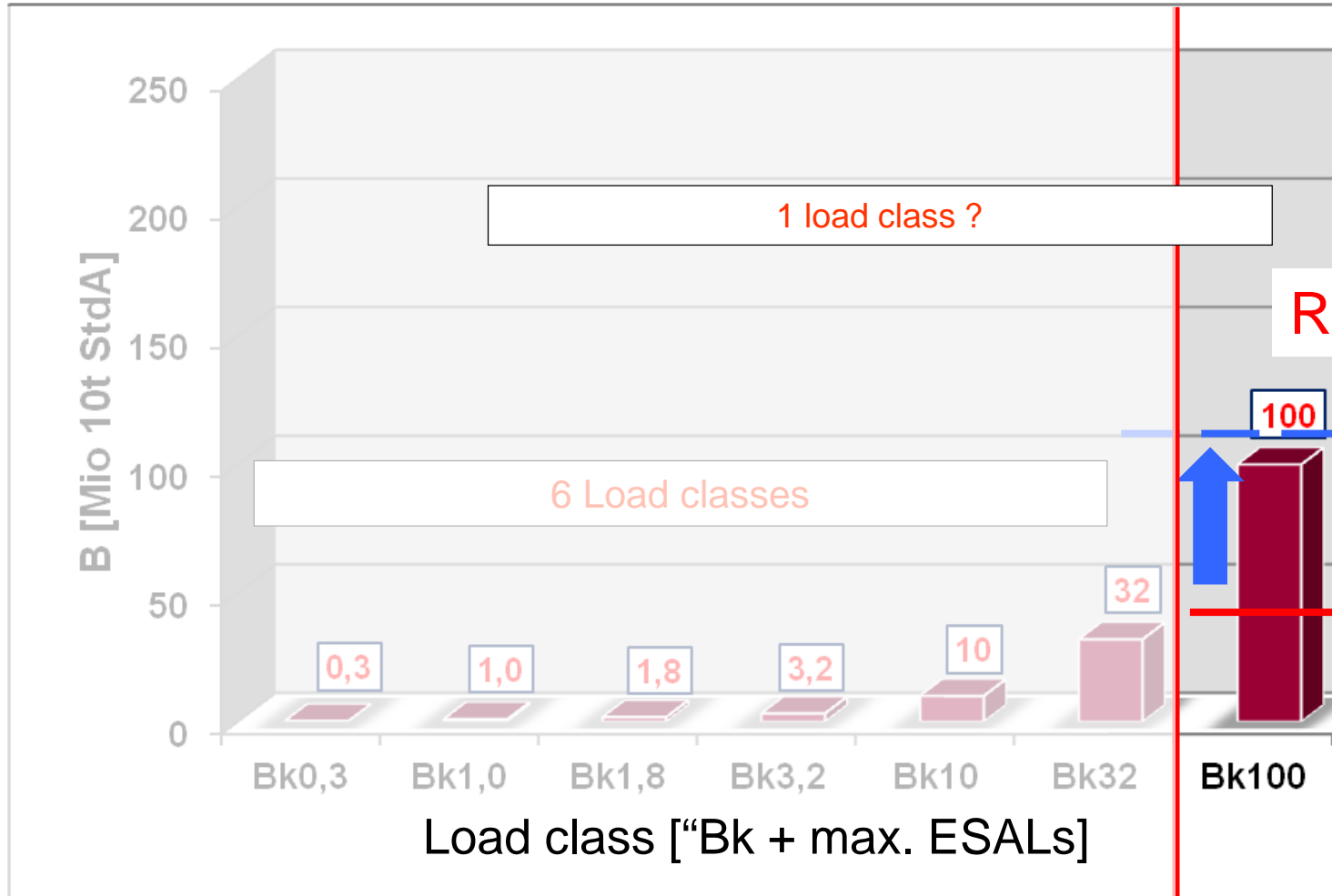


1980

2010

2040

Source: Bast 2010



Reality!

Technical background:

- permanent increasing traffic load
- climate change
- limited resources
- quality and costs of raw material

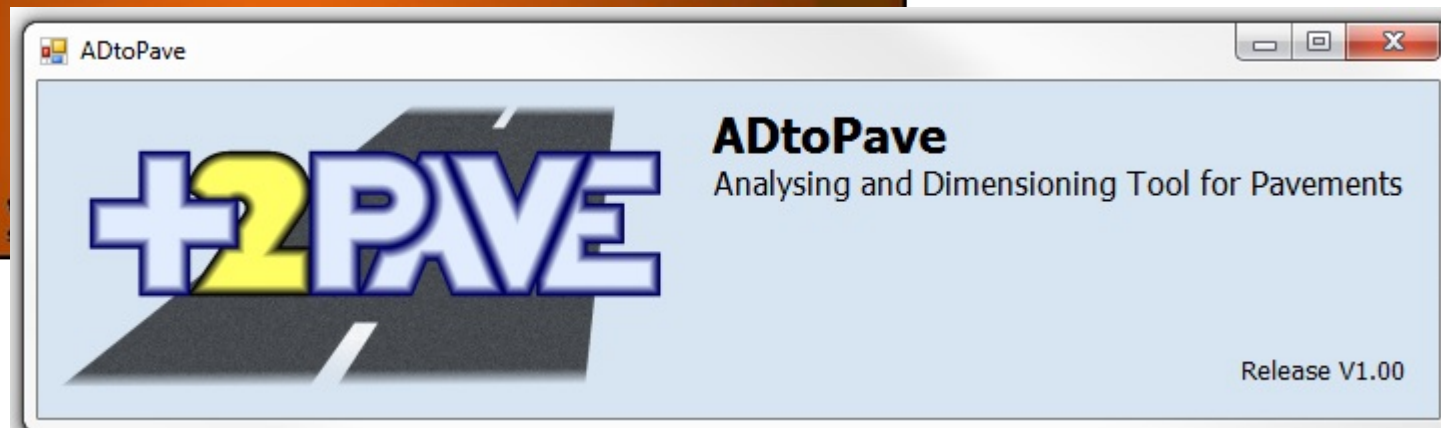
Background by contract:

- change of empirical to fundamental requirements in Germany
- introduction of functional contracts

Analytical Pavement Design Guide + Software

Pavement Design Tool

Freie Dimensionierung von Verkehrsflächenbefestigungen
Lizenziert für Prof.Dr.-Ing.F.Wellner, TU Dresden



Forschungsgesellschaft für Straßen und Verkehrswesen



Arbeitsgruppe Infrastrukturmanagement

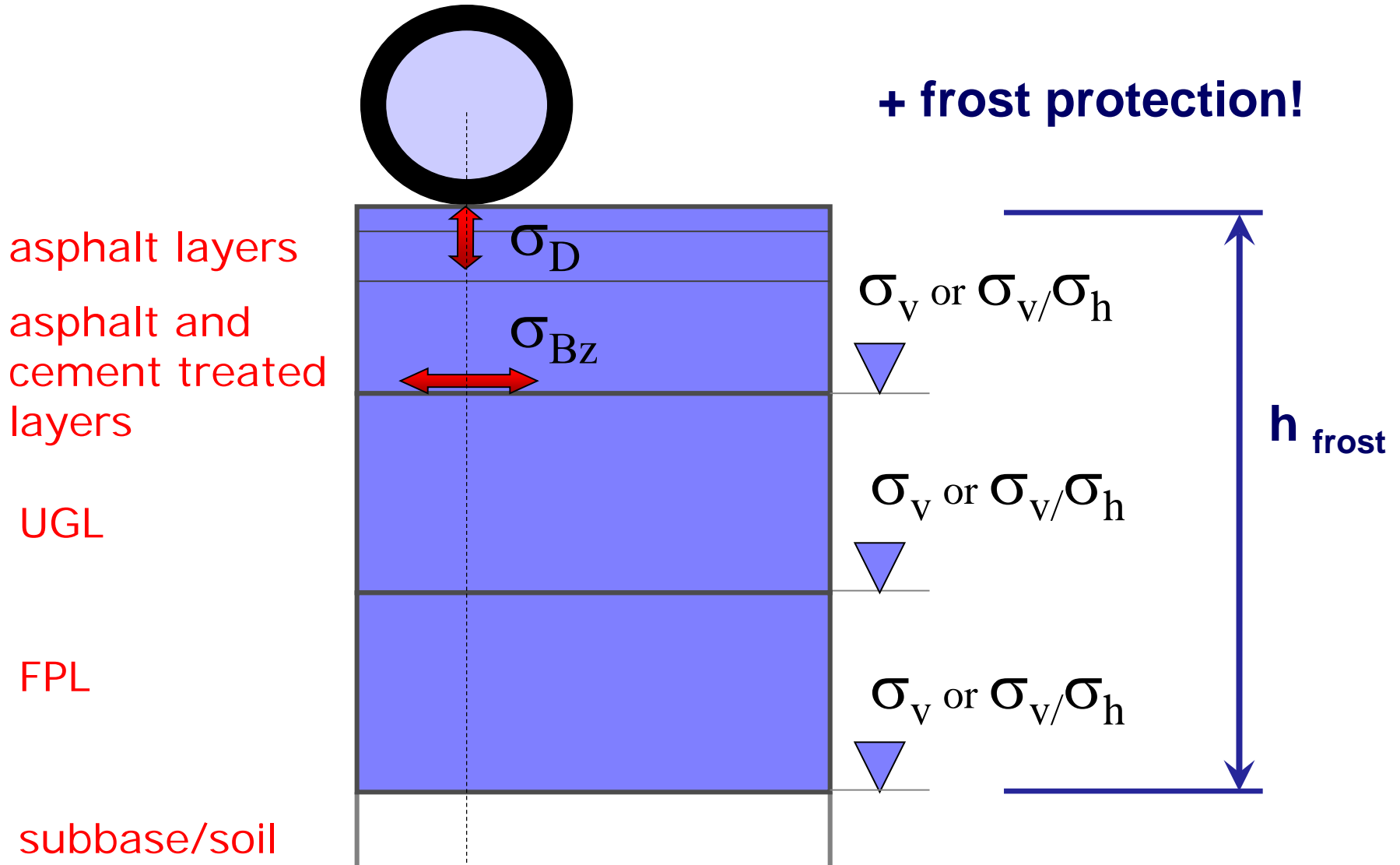
Richtlinien
für die rechnerische Dimensionierung
des Oberbaus von Verkehrsflächen
mit Asphaltdeckschicht

R 1

RDO Asphalt 09

e 2009

Release V1.00

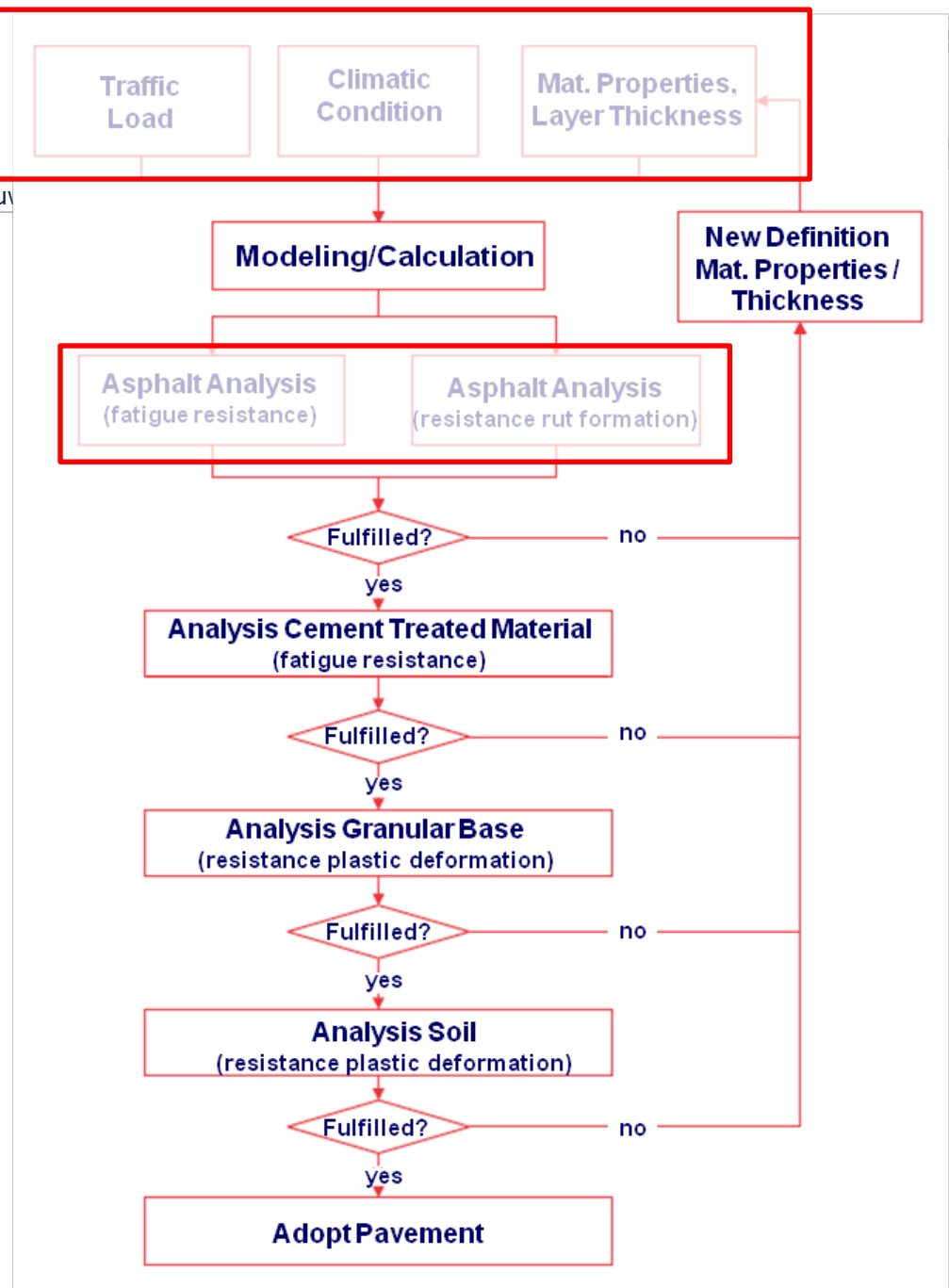


Pavement Design

Frost protection according to the RStO:

Entire thickness of pavement is depended on:

- frost penetration
- frost susceptibility of the soil in combination with
- traffic load



- **traffic load**
- climatic conditions
- stiffness
- fatigue resistance

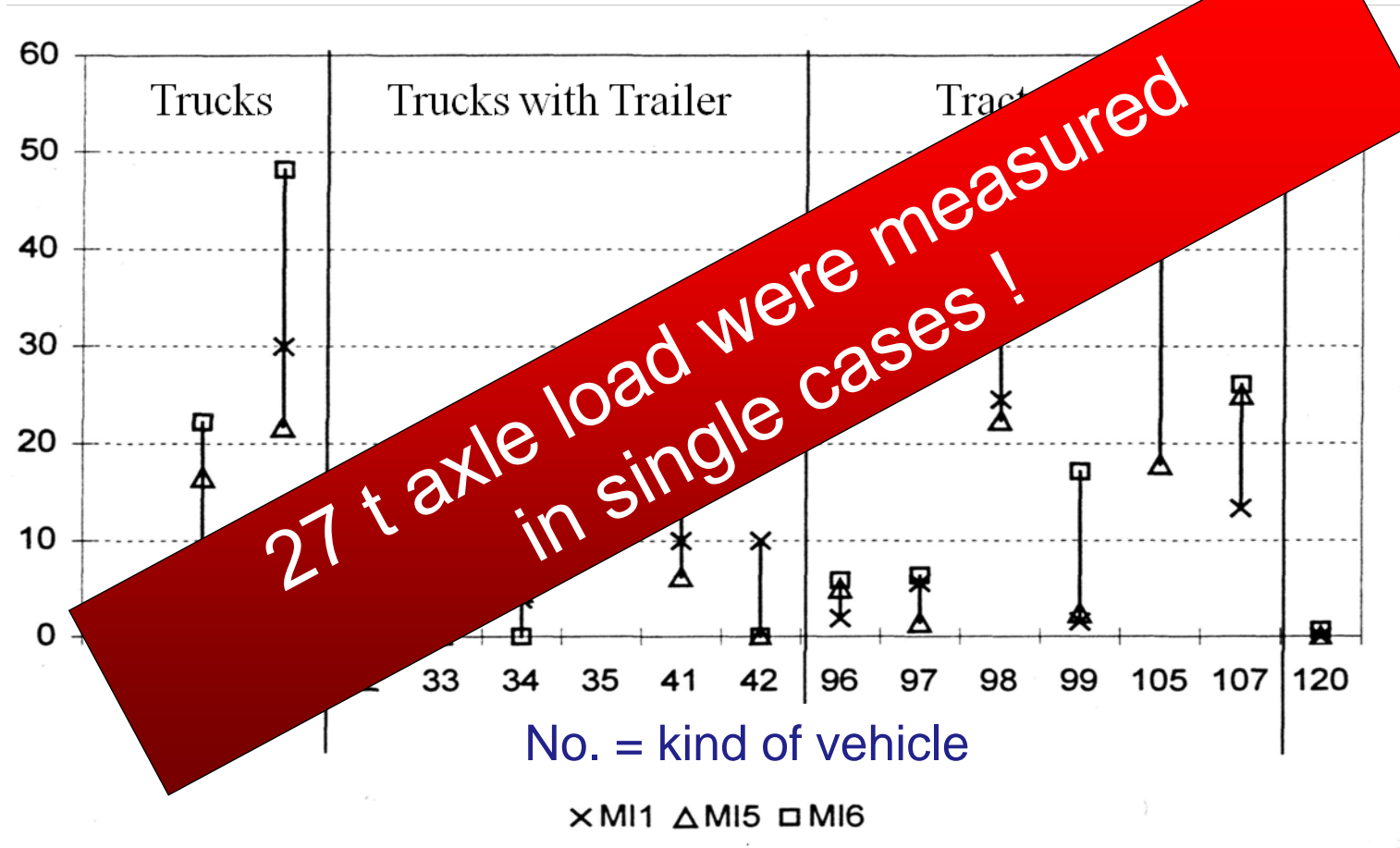
permitted traffic loads

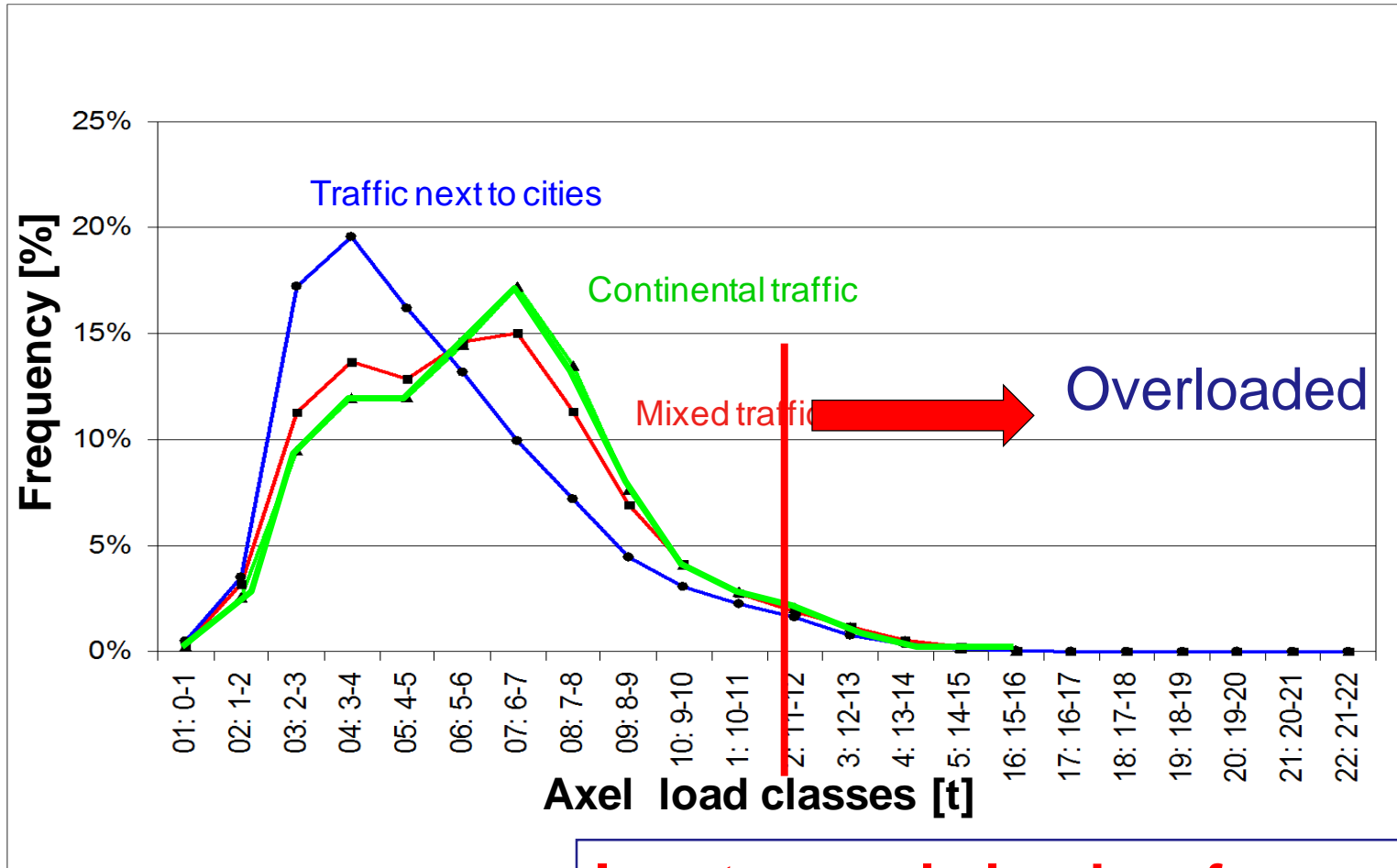
heavy traffic:	3,5 t .. 44 t weight of the vehicle 11,5 t axle load = 5,75 t wheel load
average of wheel contact pressure:	0,6 ... 0,8 MPa

future:

axle load	11,5 t ⇒ 13 t (???)
wheel contact pressure	0,8 MPa ⇒ 1,0 MPa (???)
number of car/trucks	increasing continuously

Exceed of permitted axle loads [%]

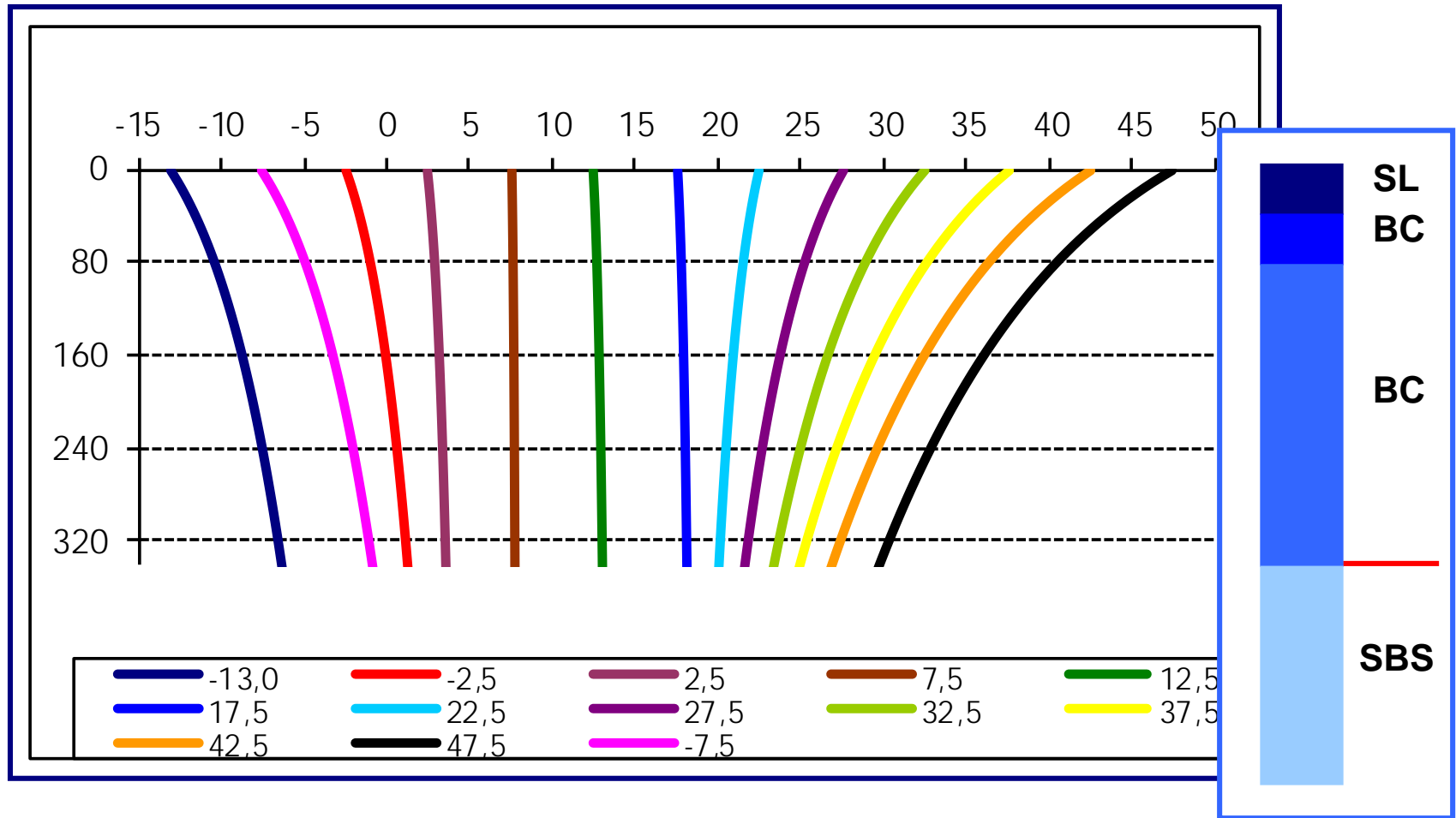


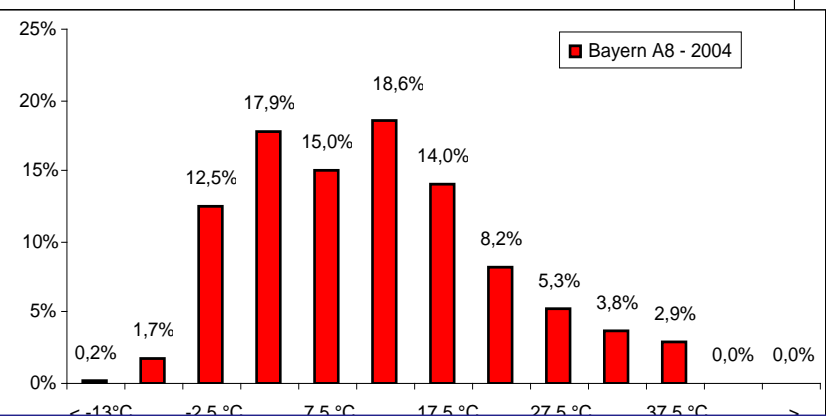
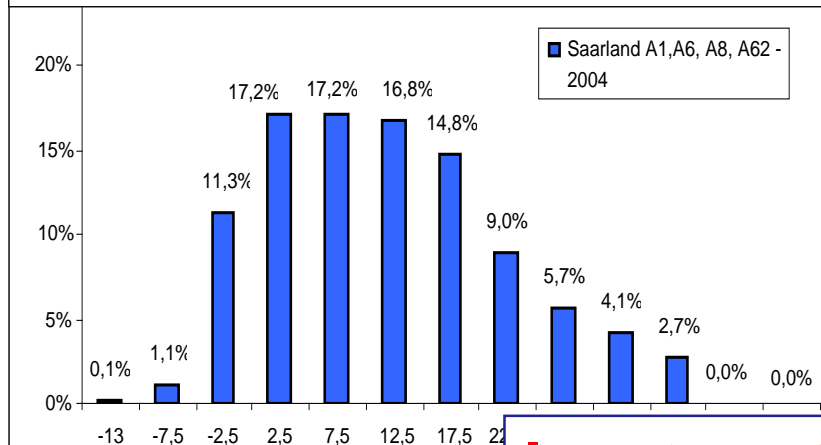
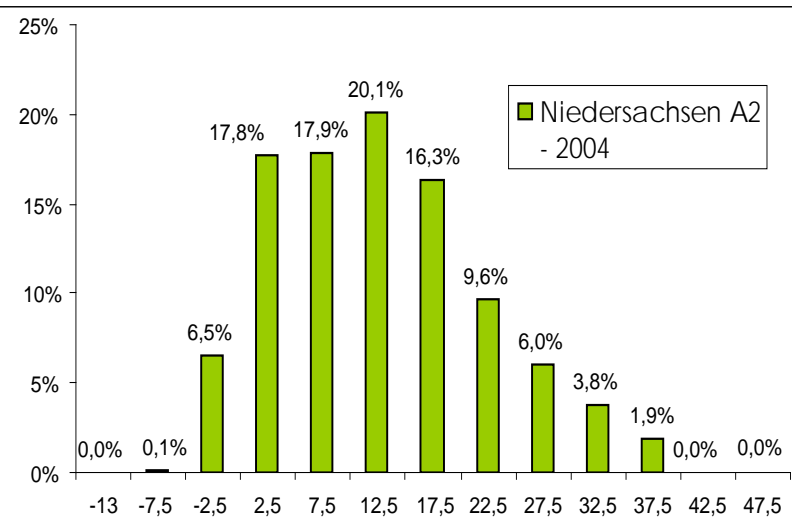
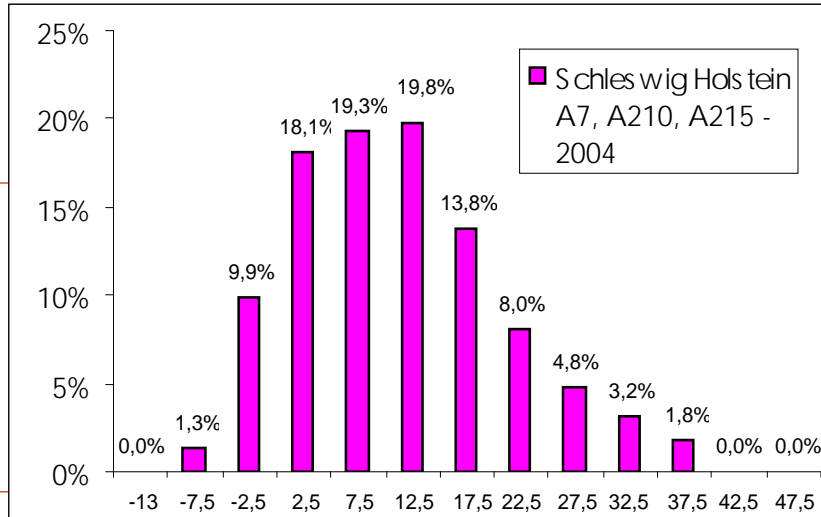


Input: axle loads + frequency of load classes

- traffic load
- climatic conditions
- stiffness
- fatigue resistance

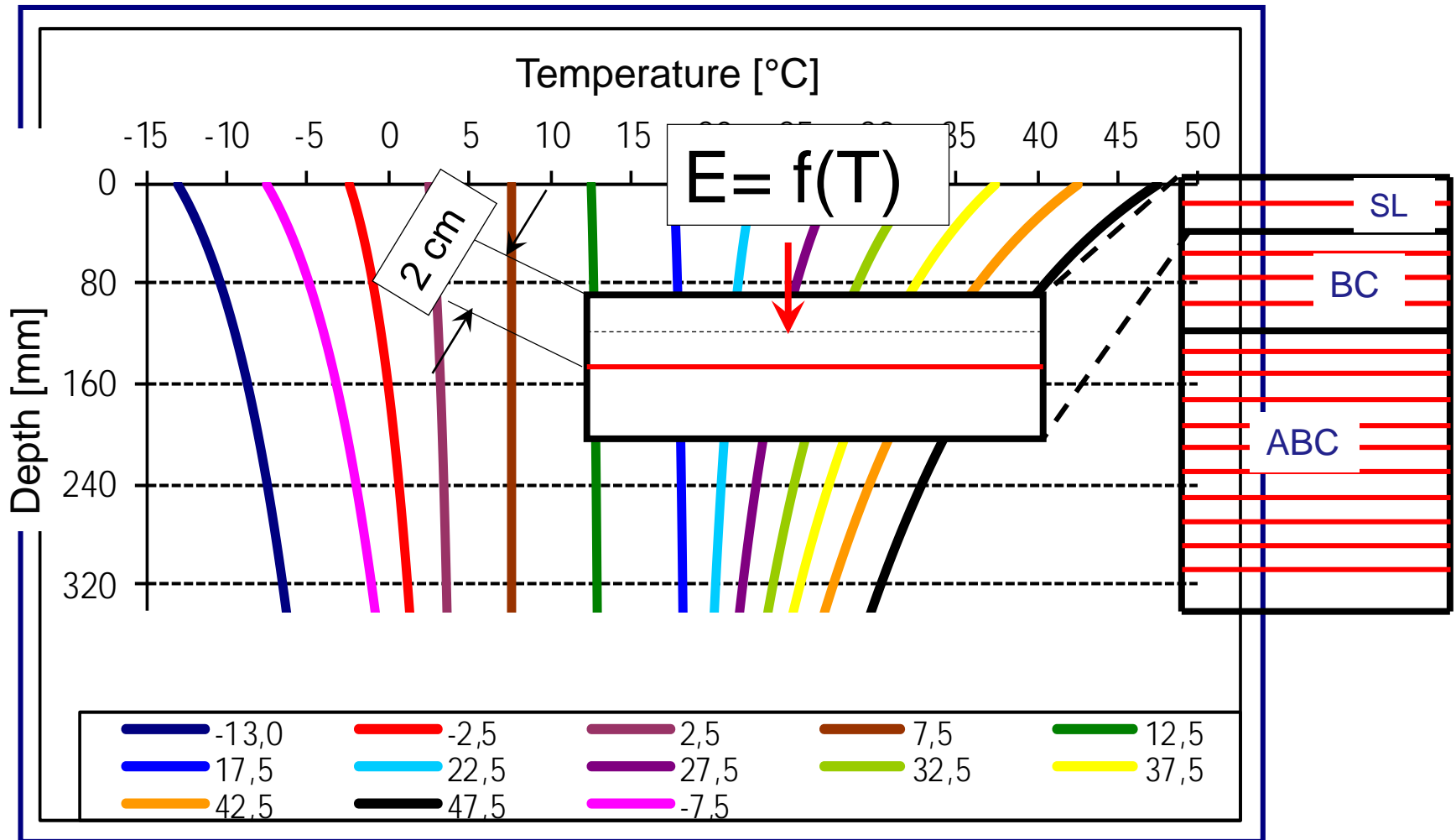




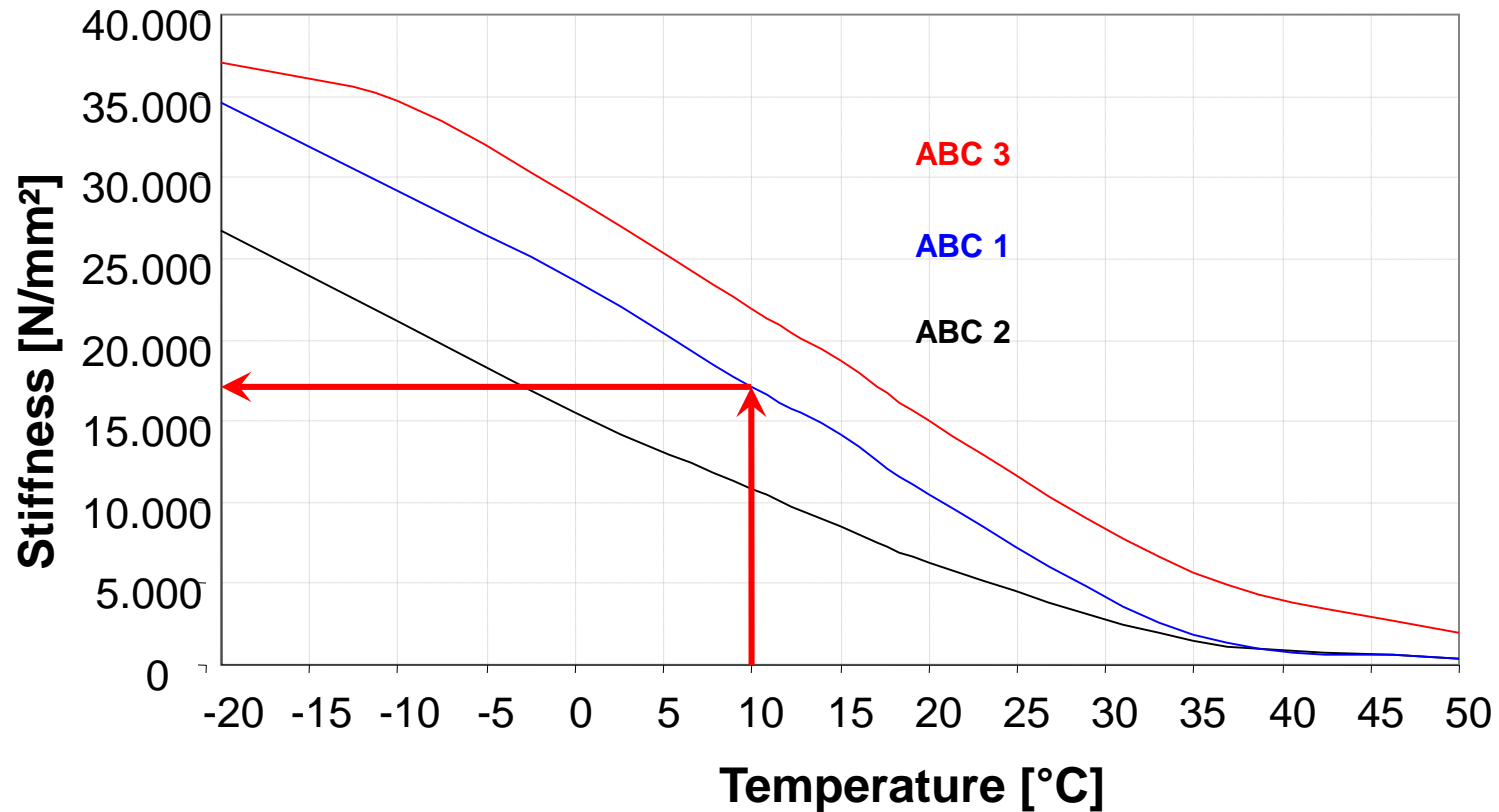


**Input: temperature functions
+ frequency of appearance**

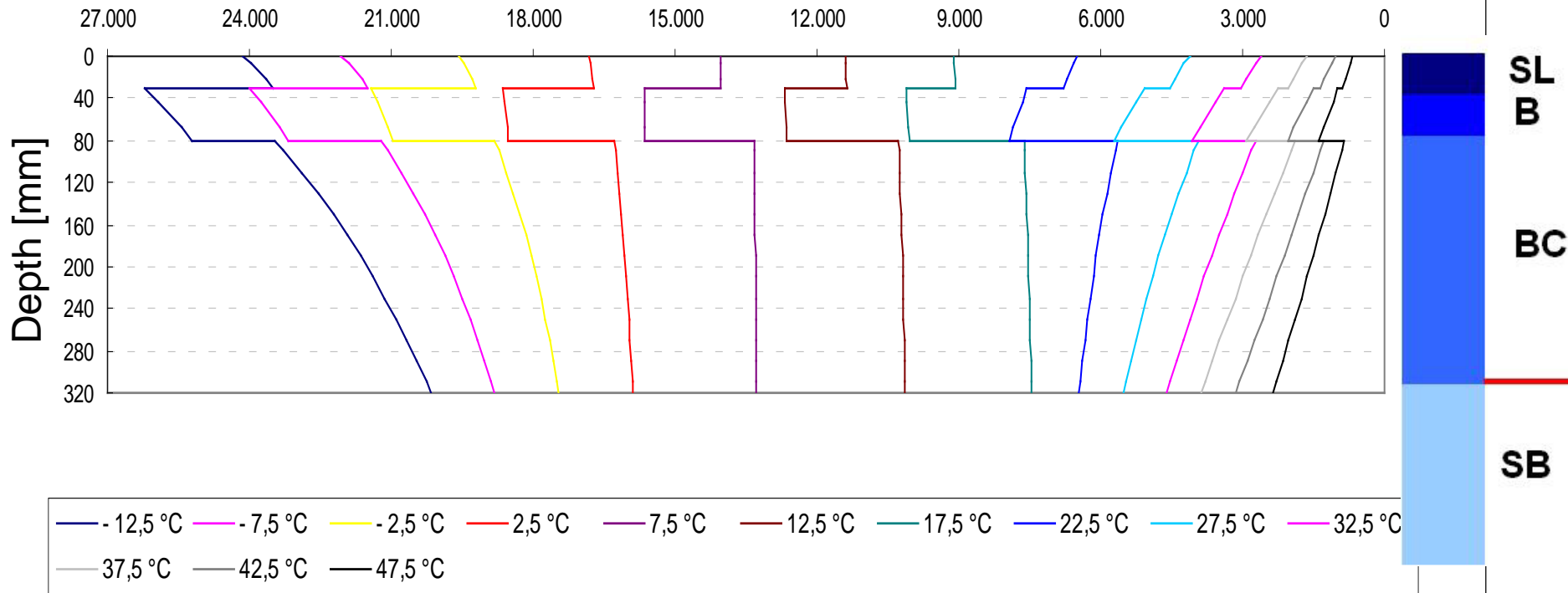
Temperature functions (examples) in the asphalt layers [MPa]



Stiffness function

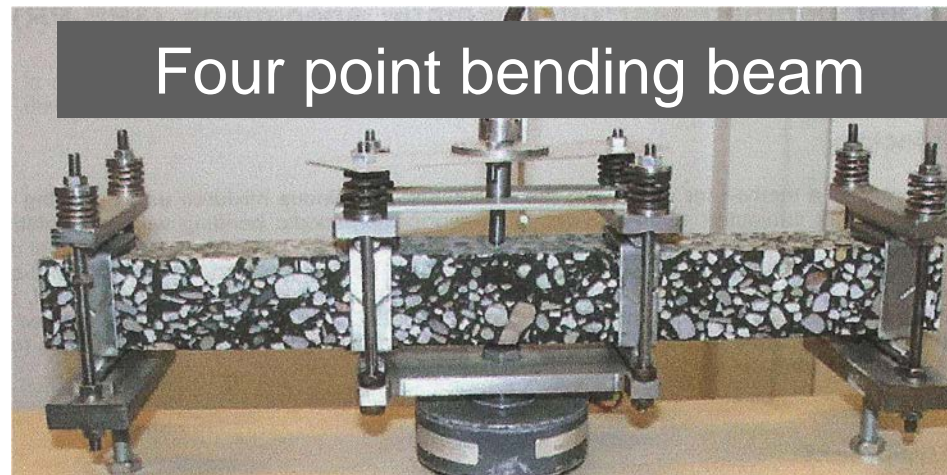
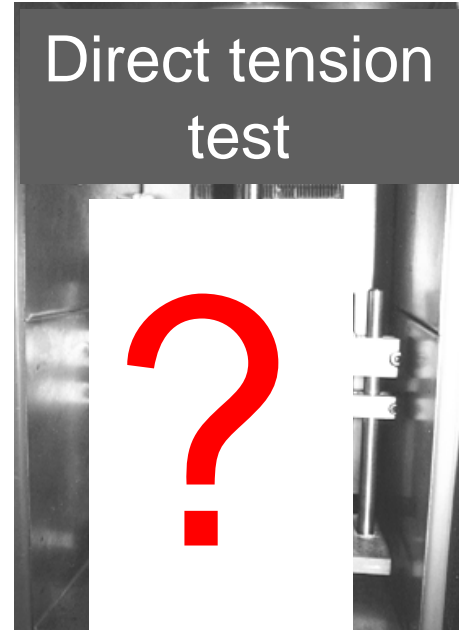
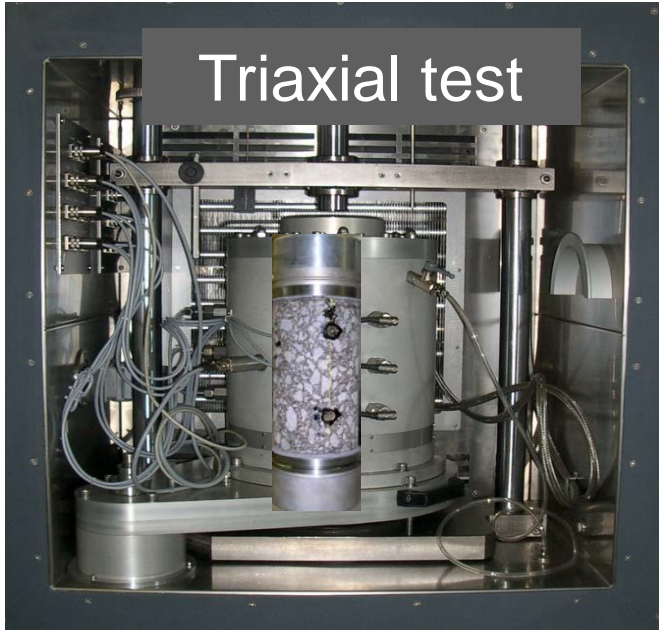


Stiffness function according to the temperature
[MPa]

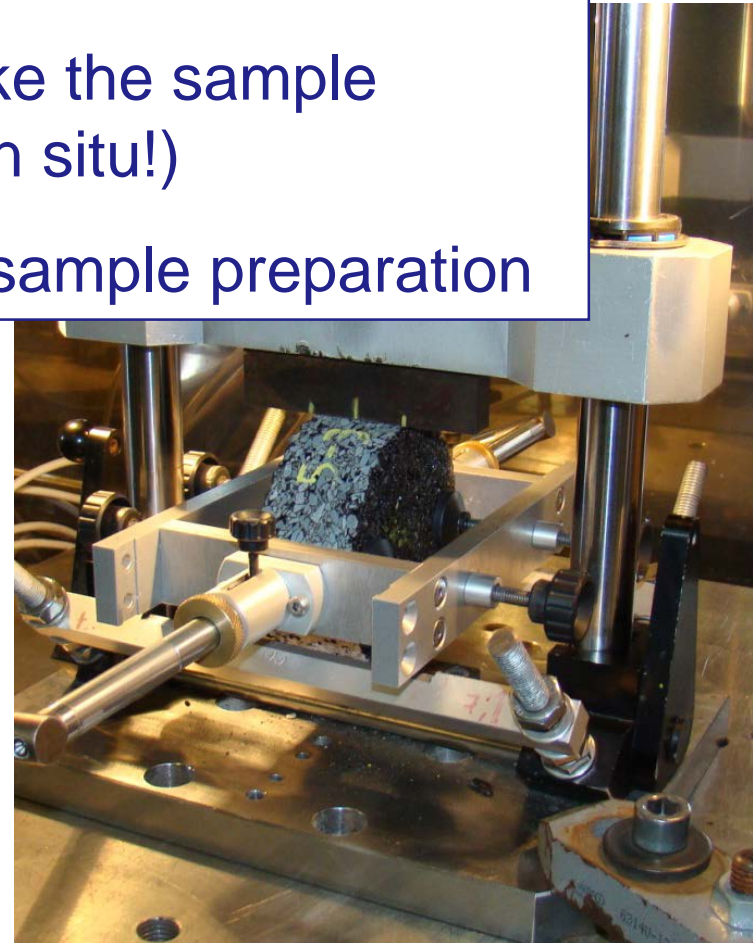
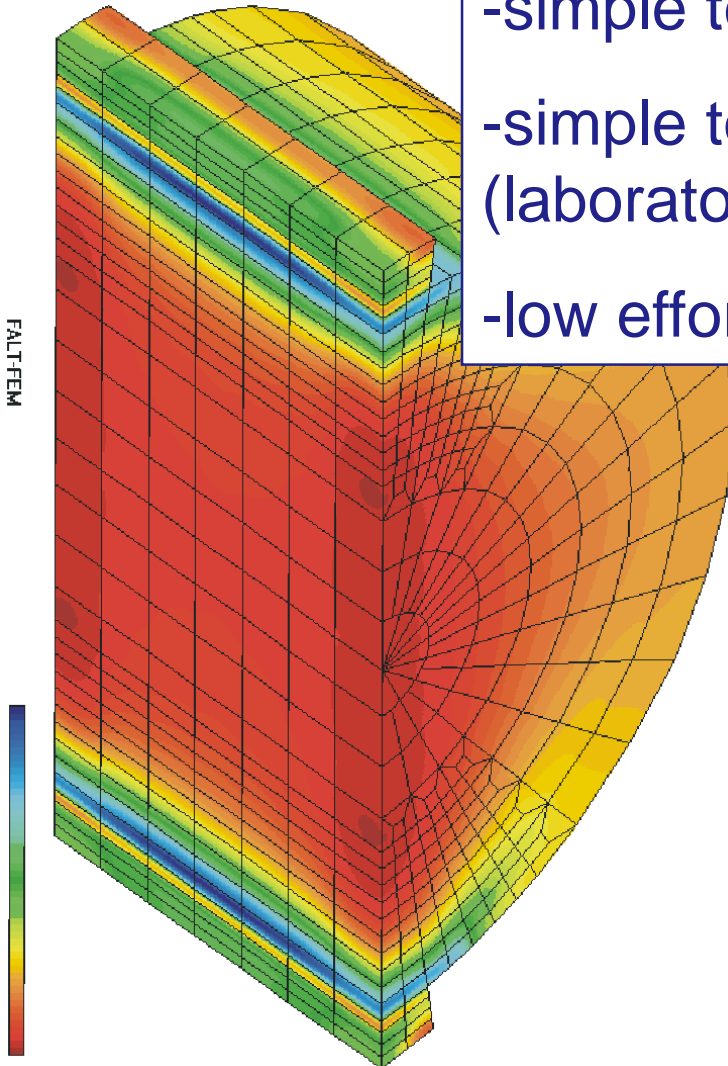


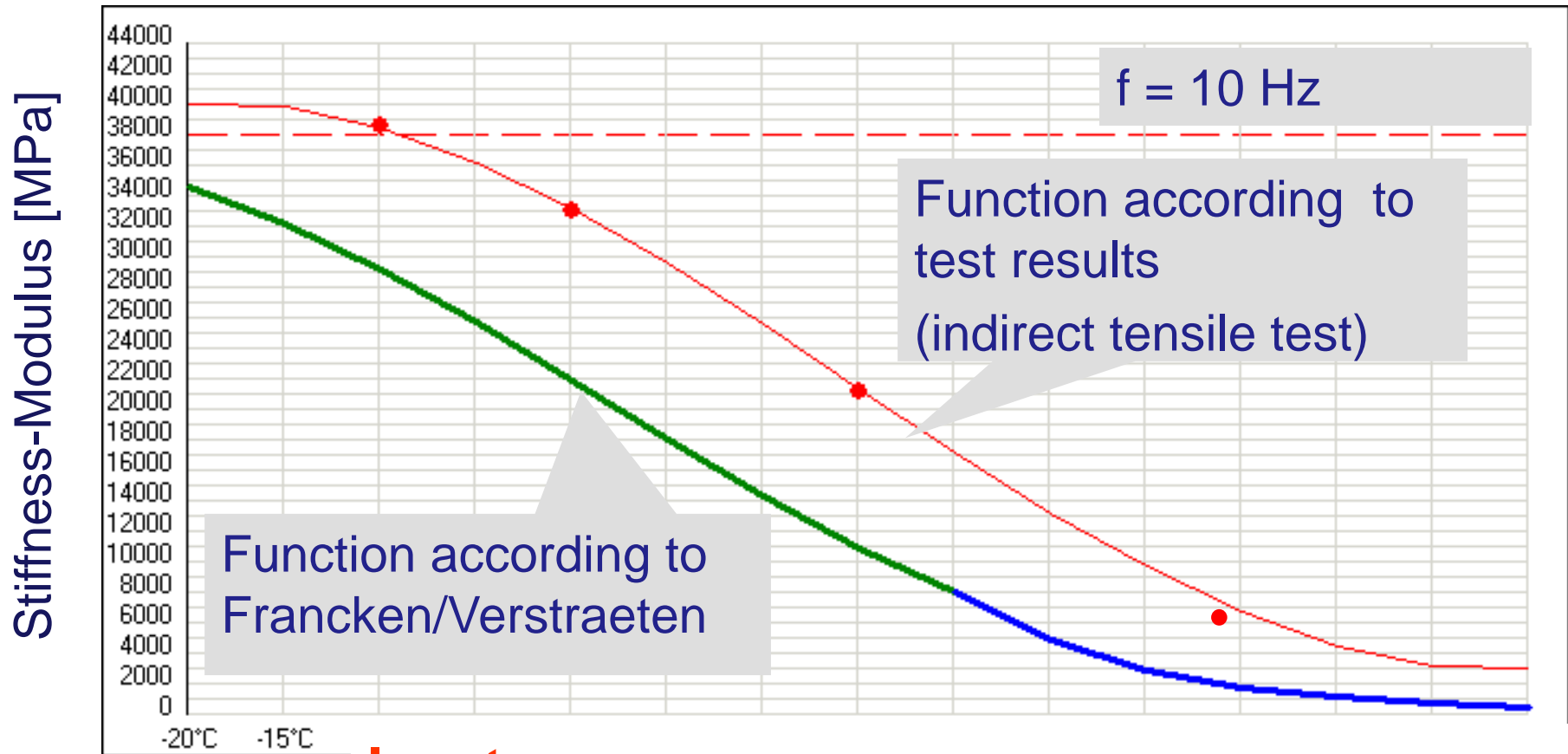
Based on this => calculation of stresses/strains

- traffic load
- climatic conditions
- **stiffness**
- **fatigue resistance**



- simple test
- simple to make the sample (laboratory + in situ!)
- low effort for sample preparation

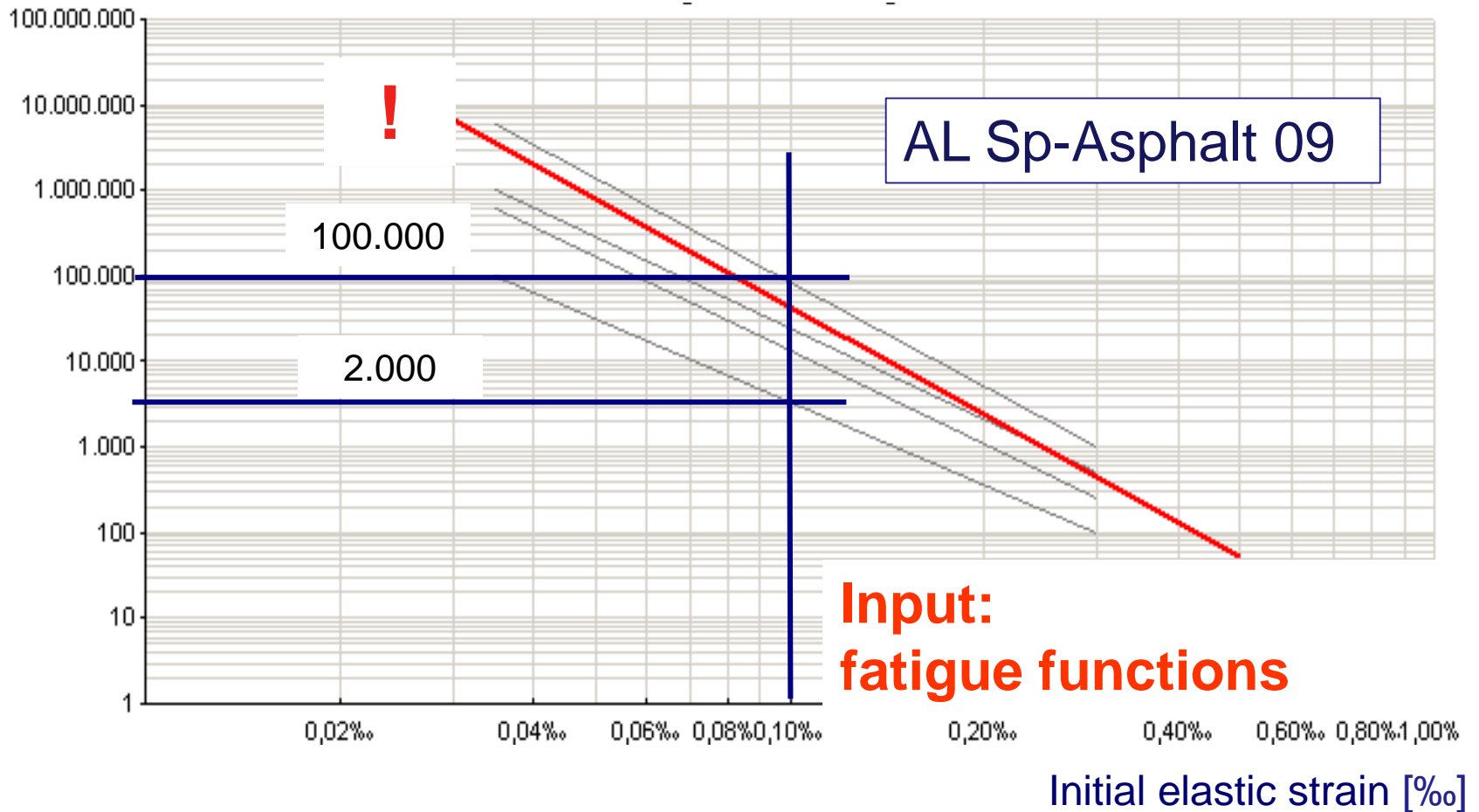




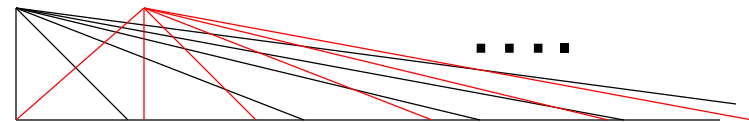
Input:
Stiffness modulus - temperature functions

Load cycles

Fatigue functions



13 temperature conditions I II III IV V...XIII



10...20 load classes I II III IV V ... X

13 (...200) * 10 (10...20) = 130 (4000) strain conditions

evaluation: permitted $N = F(\text{calculated } \varepsilon)$
per condition

accumulation: Miner's law

(test results)

$$\text{per } N = f(\text{exp } \sigma, \gamma)$$

(input data's)

$$\text{exp } N$$

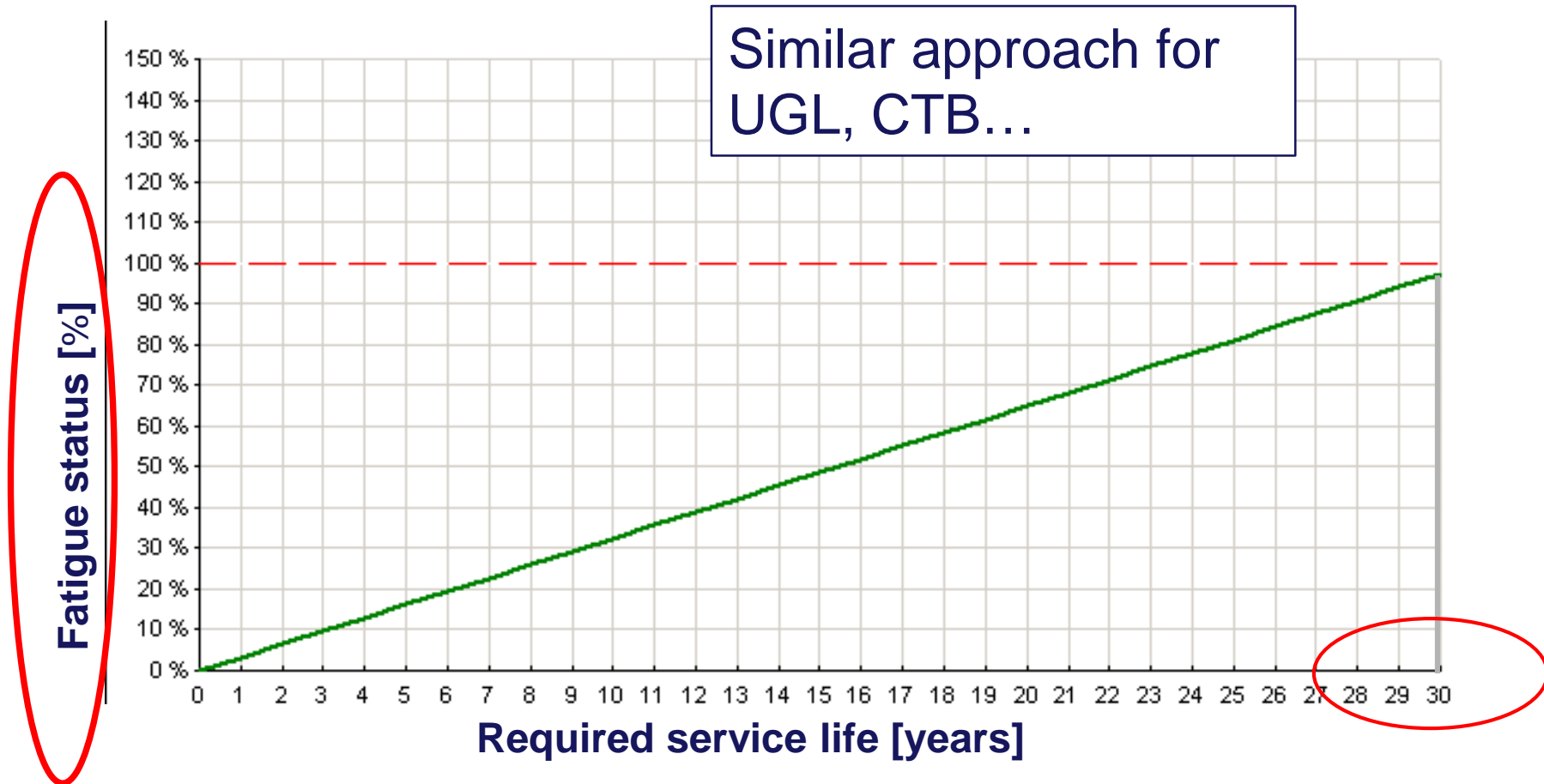
$$\sum_{\text{Miner}} = \frac{\text{exp } N_1}{\text{per } N_1} + \frac{\text{exp } N_2}{\text{per } N_2} + \dots + \frac{\text{exp } N_n}{\text{per } N_n} \leq 1$$

exp – expected

per - permitted

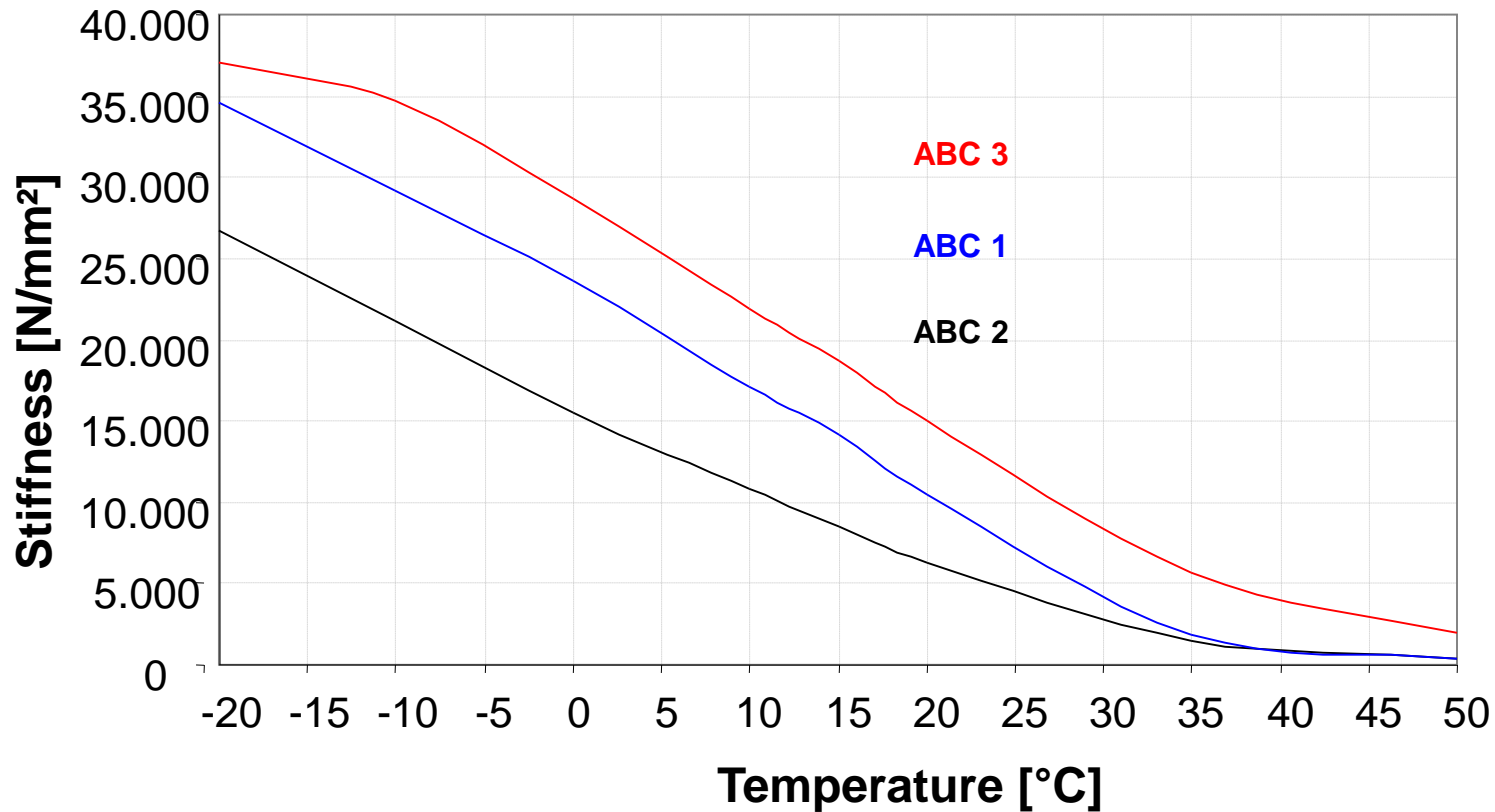
Miner's law

1*100 = 100% = „Fatigue status“



Design criterion „Fatigue“

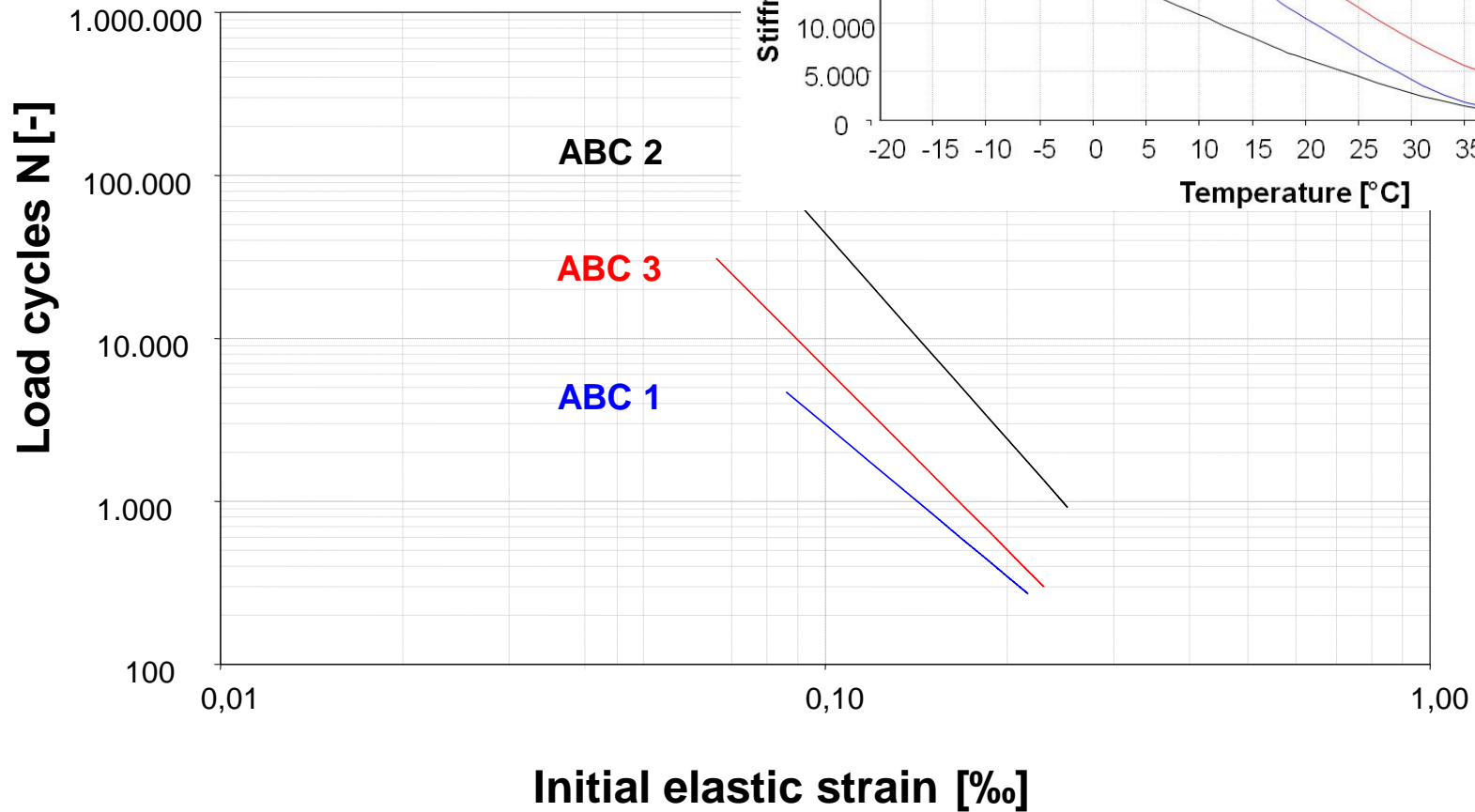
Stiffness – determined by indirect tensile test



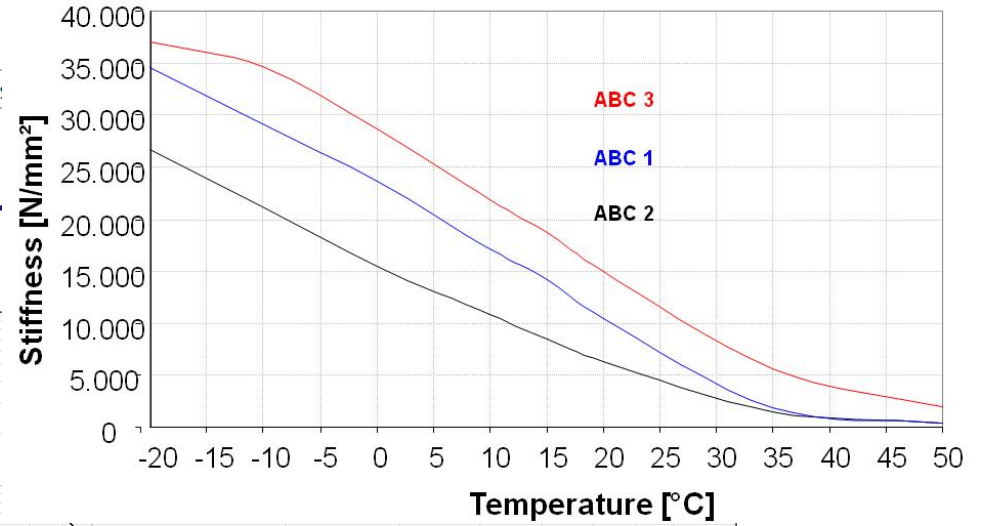
Example 1

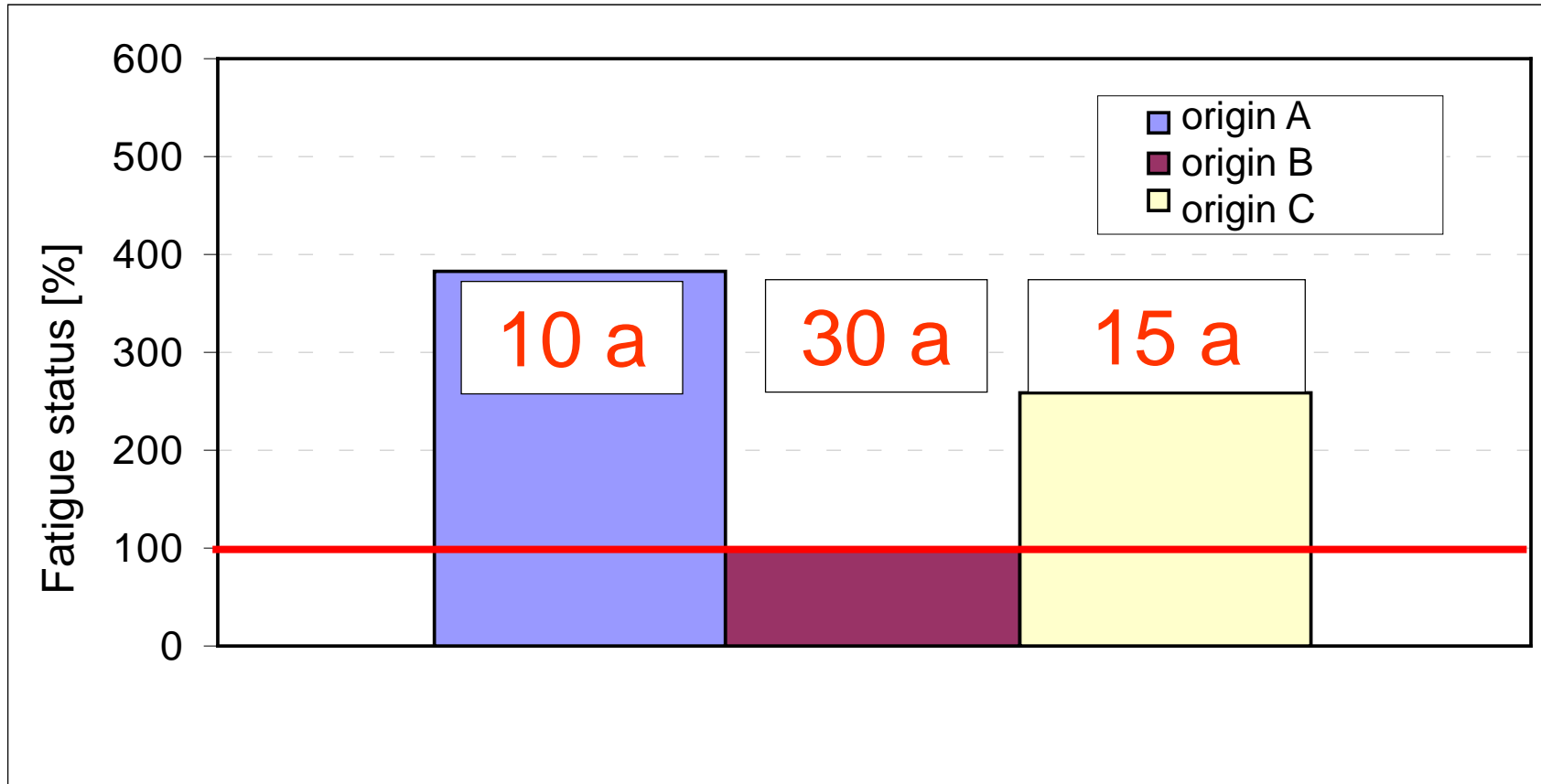
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Fatigue functions - detern

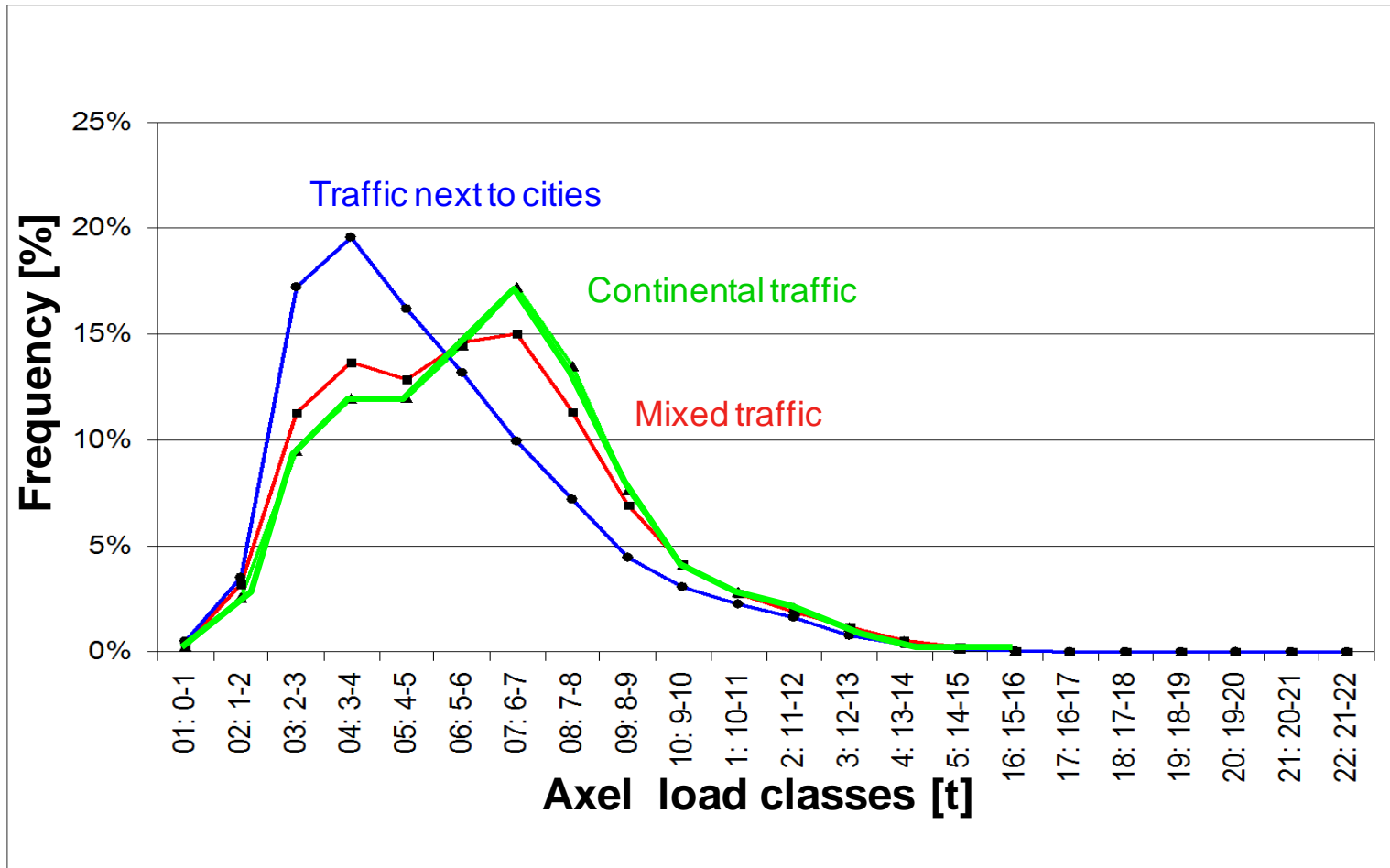


Stiffness – determined by indirect tensile test



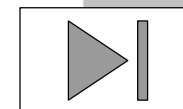
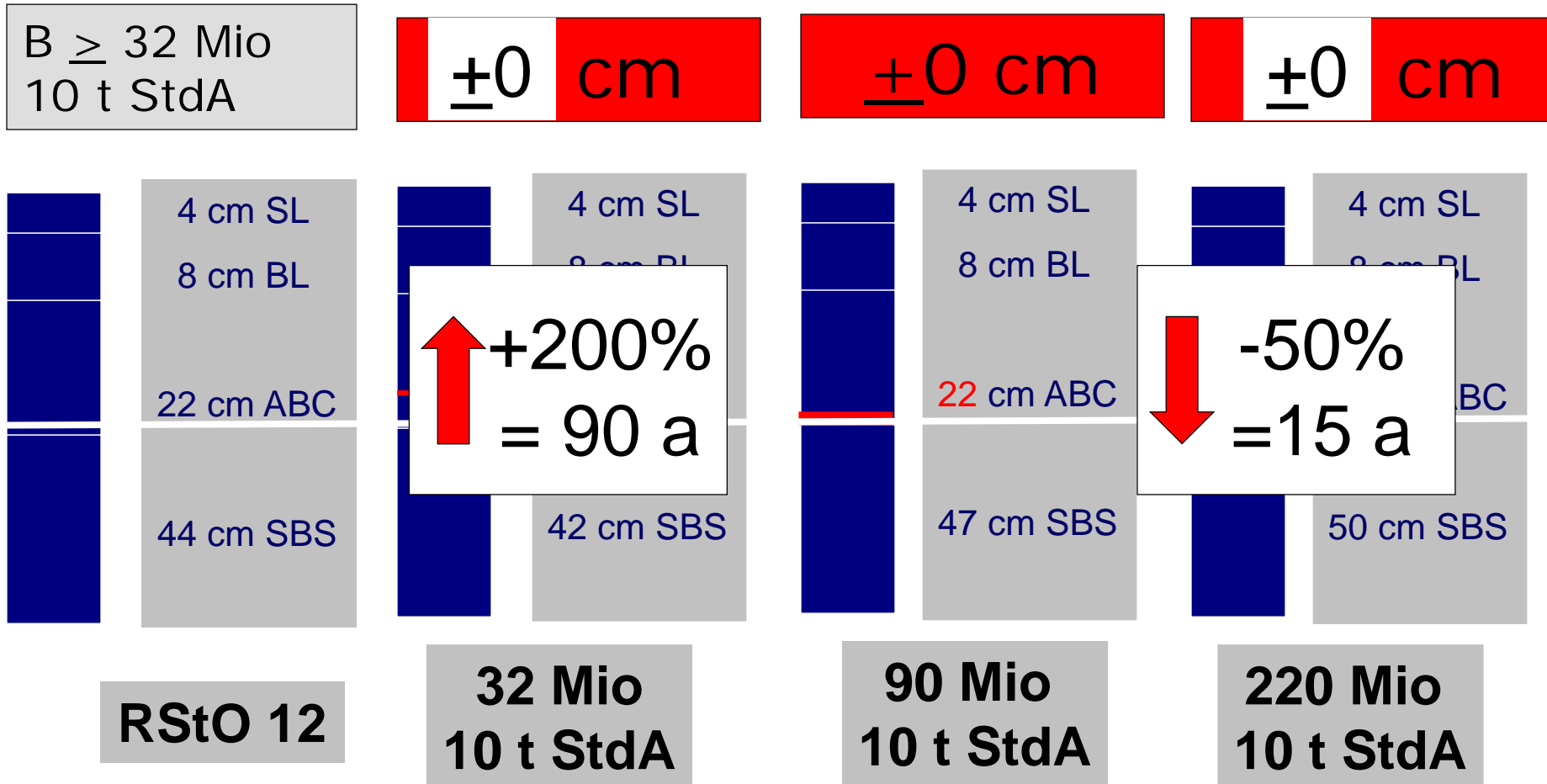


Level of fatigue of asphalt base course material with the **same stone material and sieving curve** and Bitumen **50/70** with **different origin**





Effect on service life?



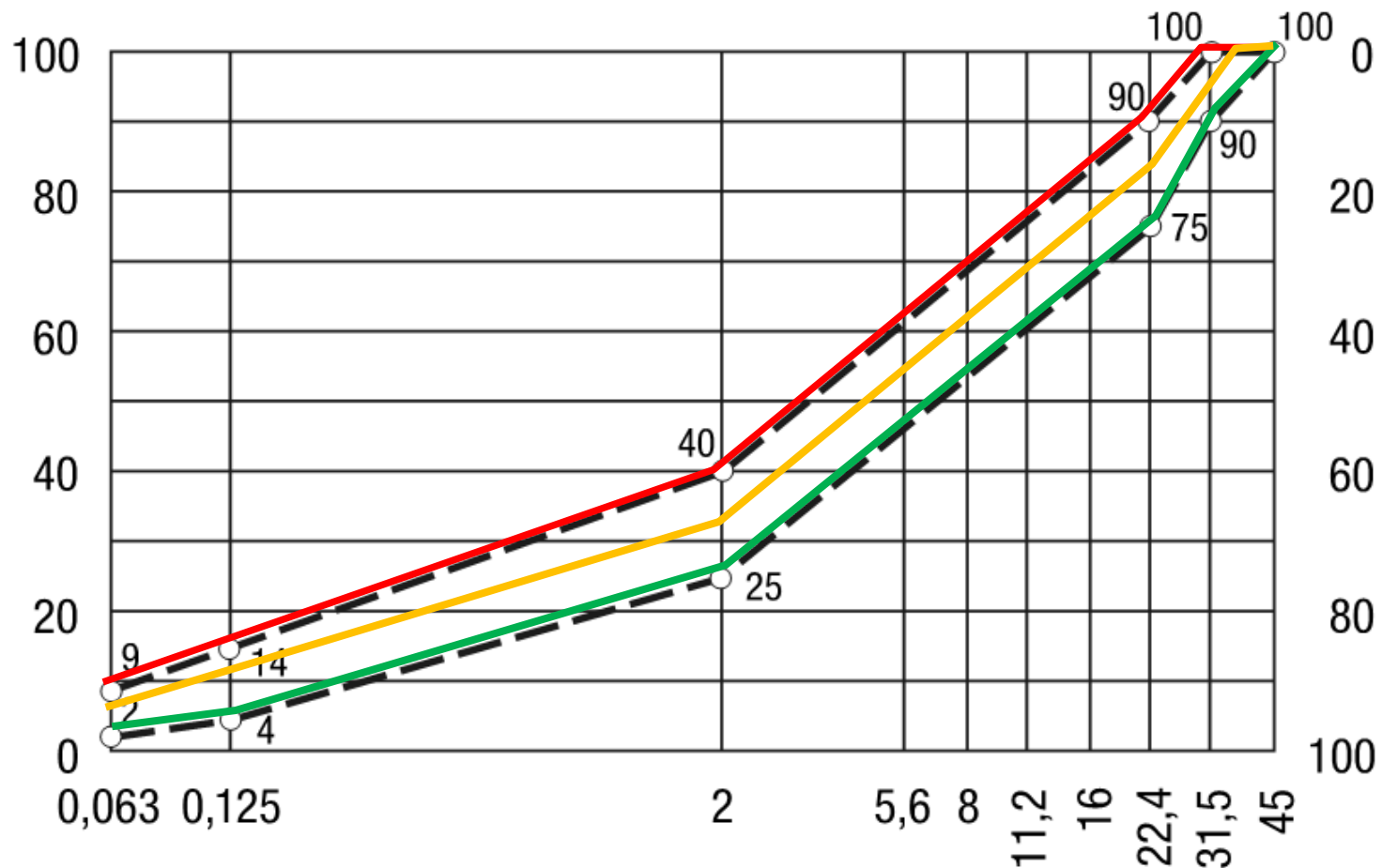
Example 3

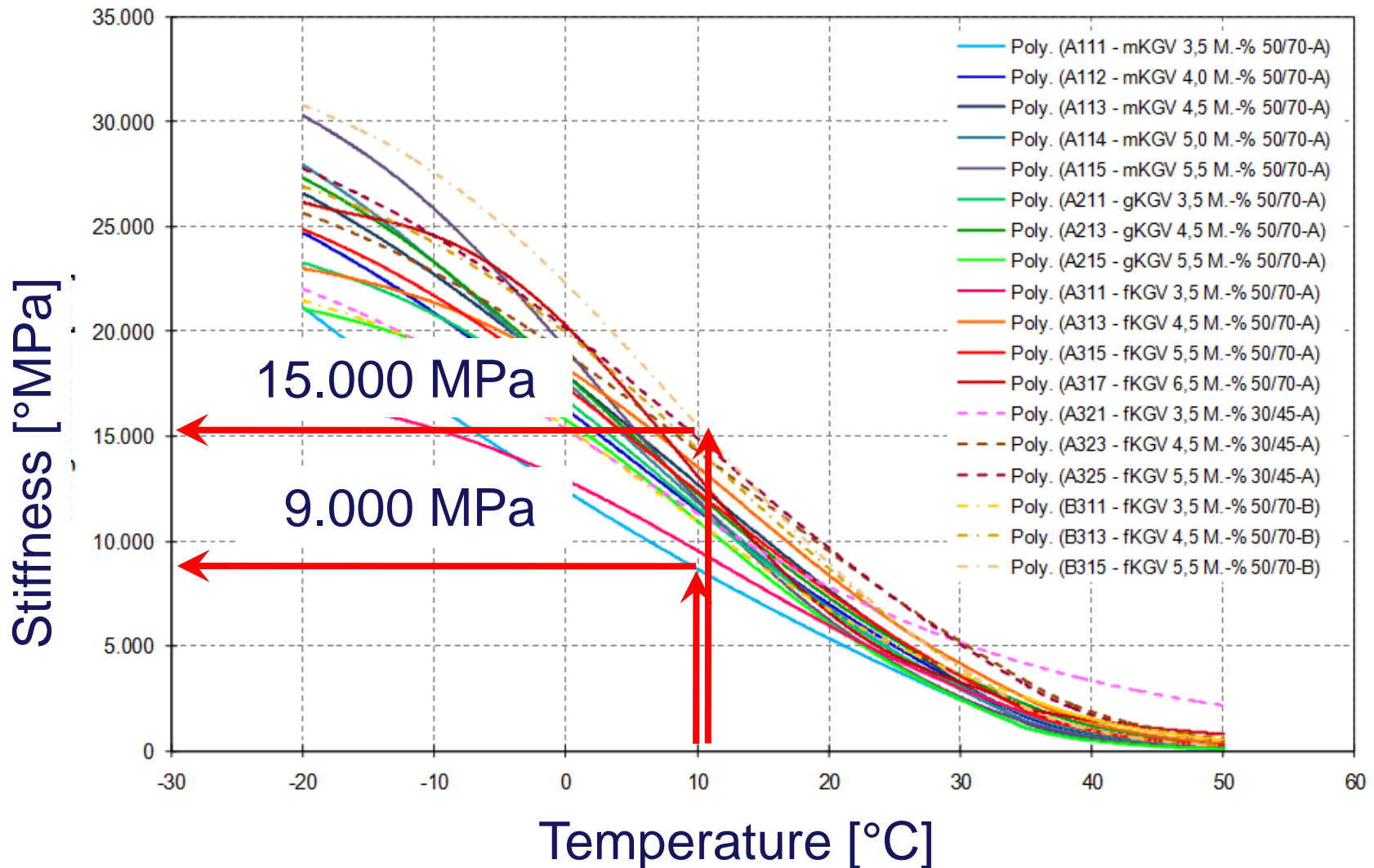
Performance based mix design method

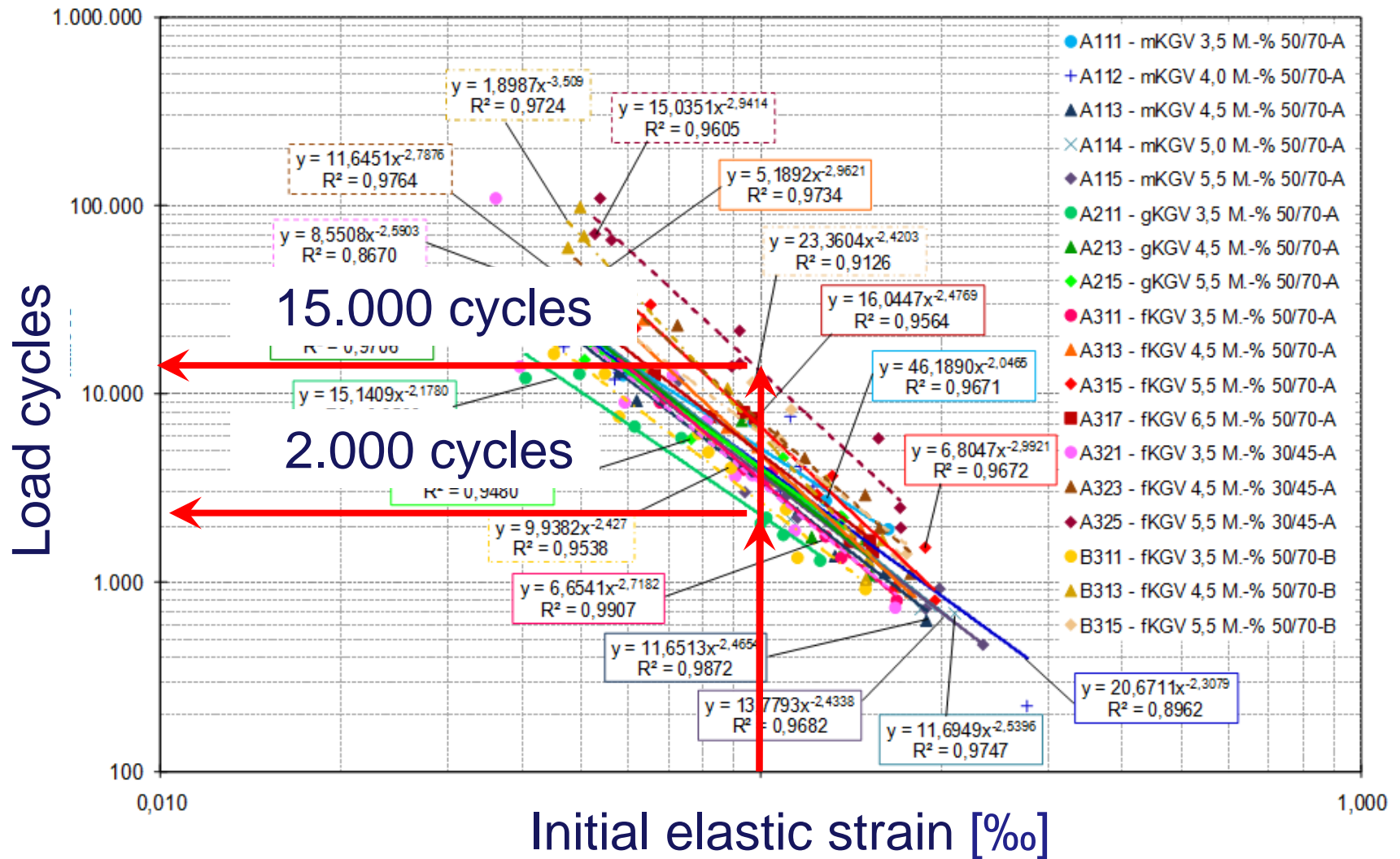
- Number of asphalt mixes -

Grading curve	Bitumen	Producer	No. of Bitumen content
medium	50/70	A	5
coarse			3
fine			30/45
	50/70	B	3
Σ Asphalt mixes			18

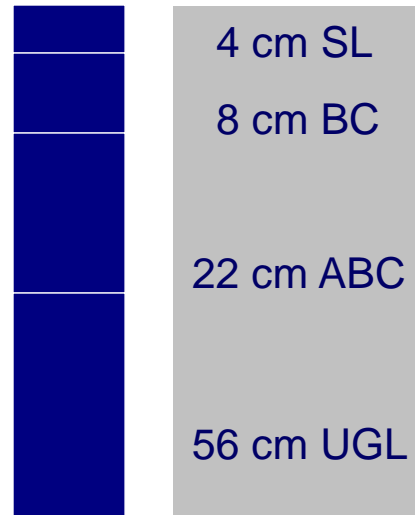
„fine“ „medium“ „coarse“







B = 32 Mio ESALs



RStO 12

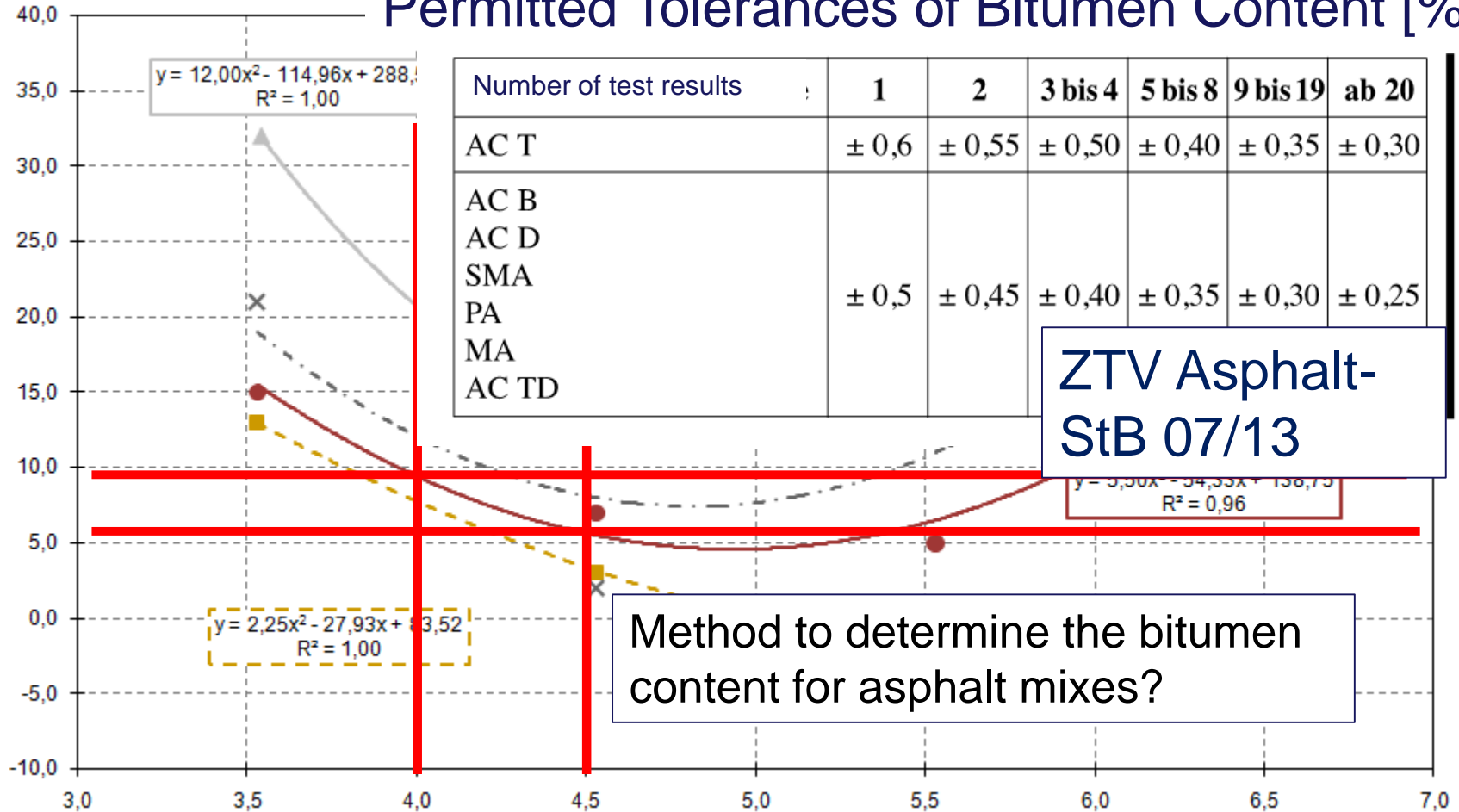


Permitted Tolerances of Bitumen Content [%]

Number of test results	1	2	3 bis 4	5 bis 8	9 bis 19	ab 20
AC T	± 0,6	± 0,55	± 0,50	± 0,40	± 0,35	± 0,30
AC B						
AC D						
SMA						
PA	± 0,5	± 0,45	± 0,40	± 0,35	± 0,30	± 0,25
MA						
AC TD						

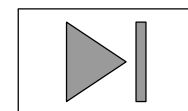
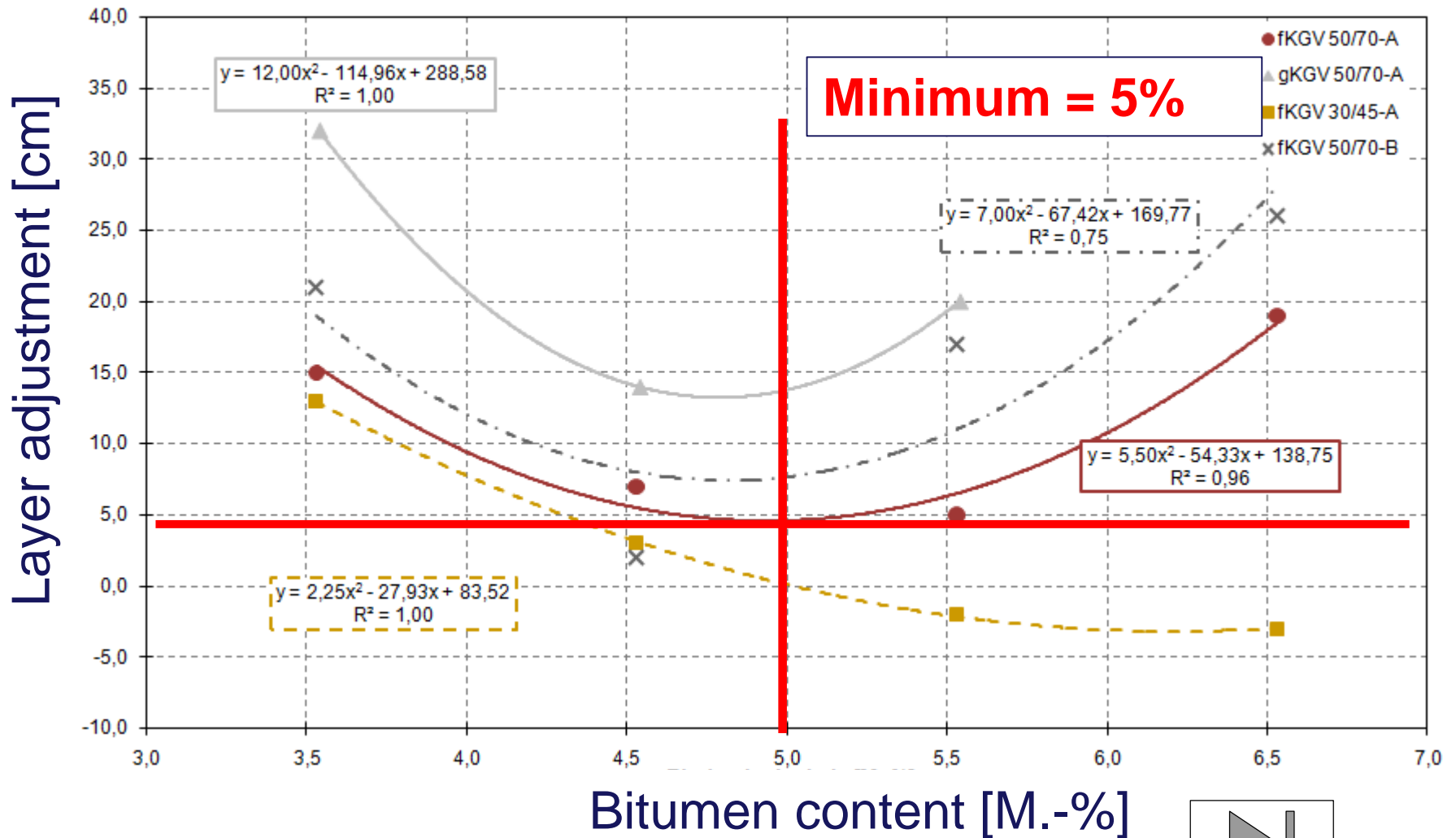
ZTV Asphalt-StB 07/13

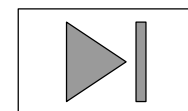
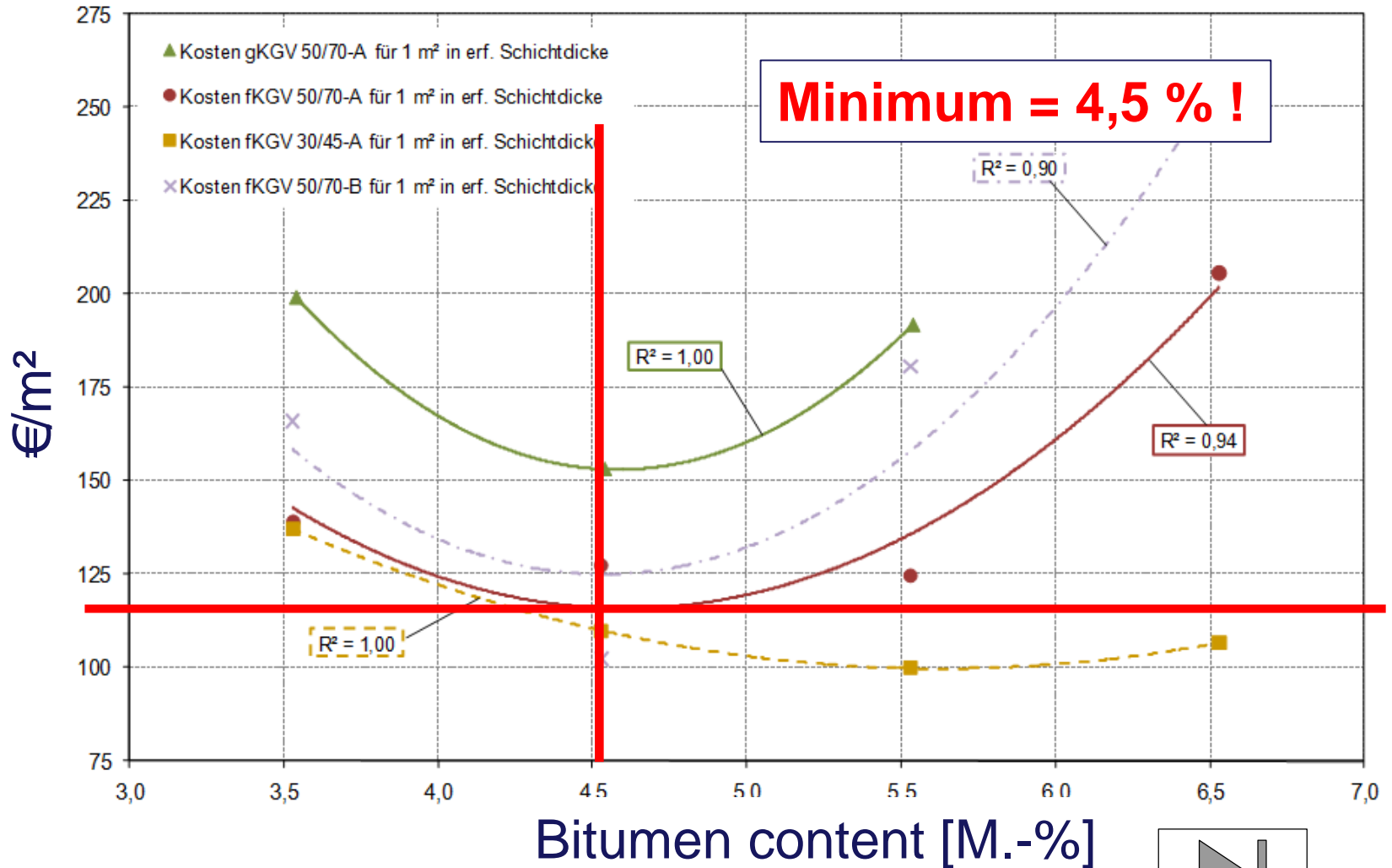
Layer adjustmen [cm]



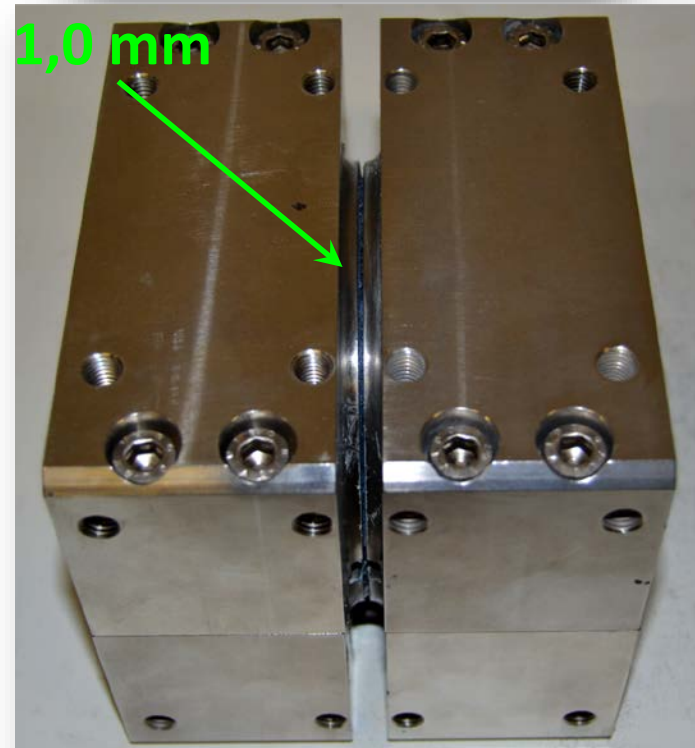
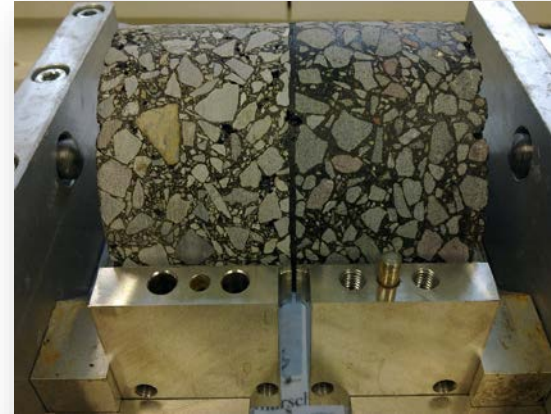
Method to determine the bitumen content for asphalt mixes?

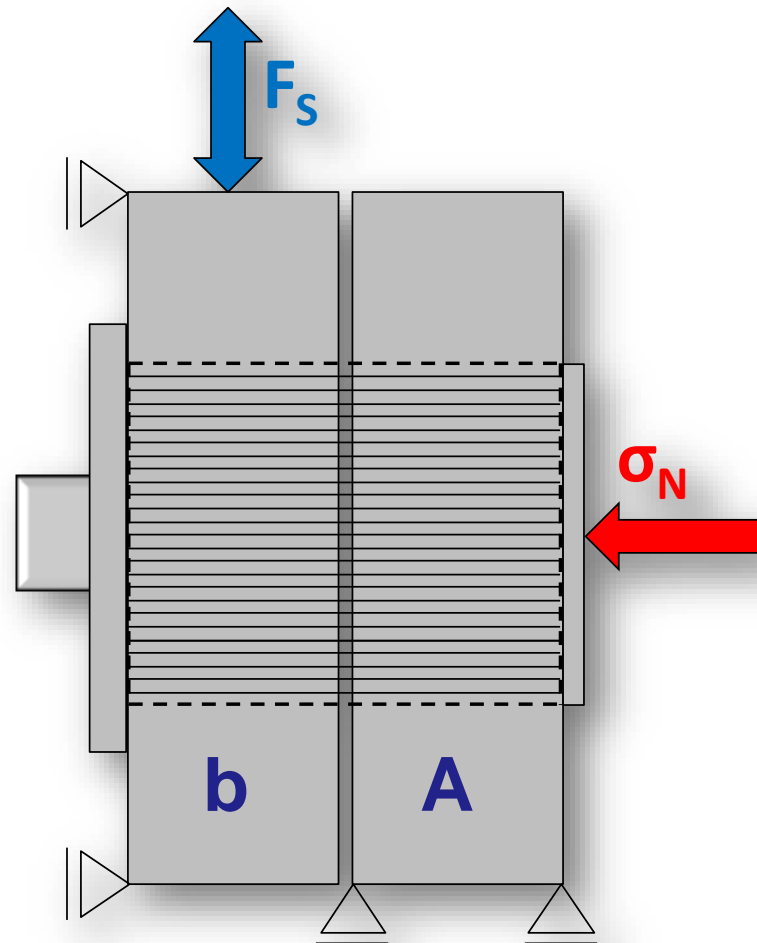
Bitumen content [M.-%]

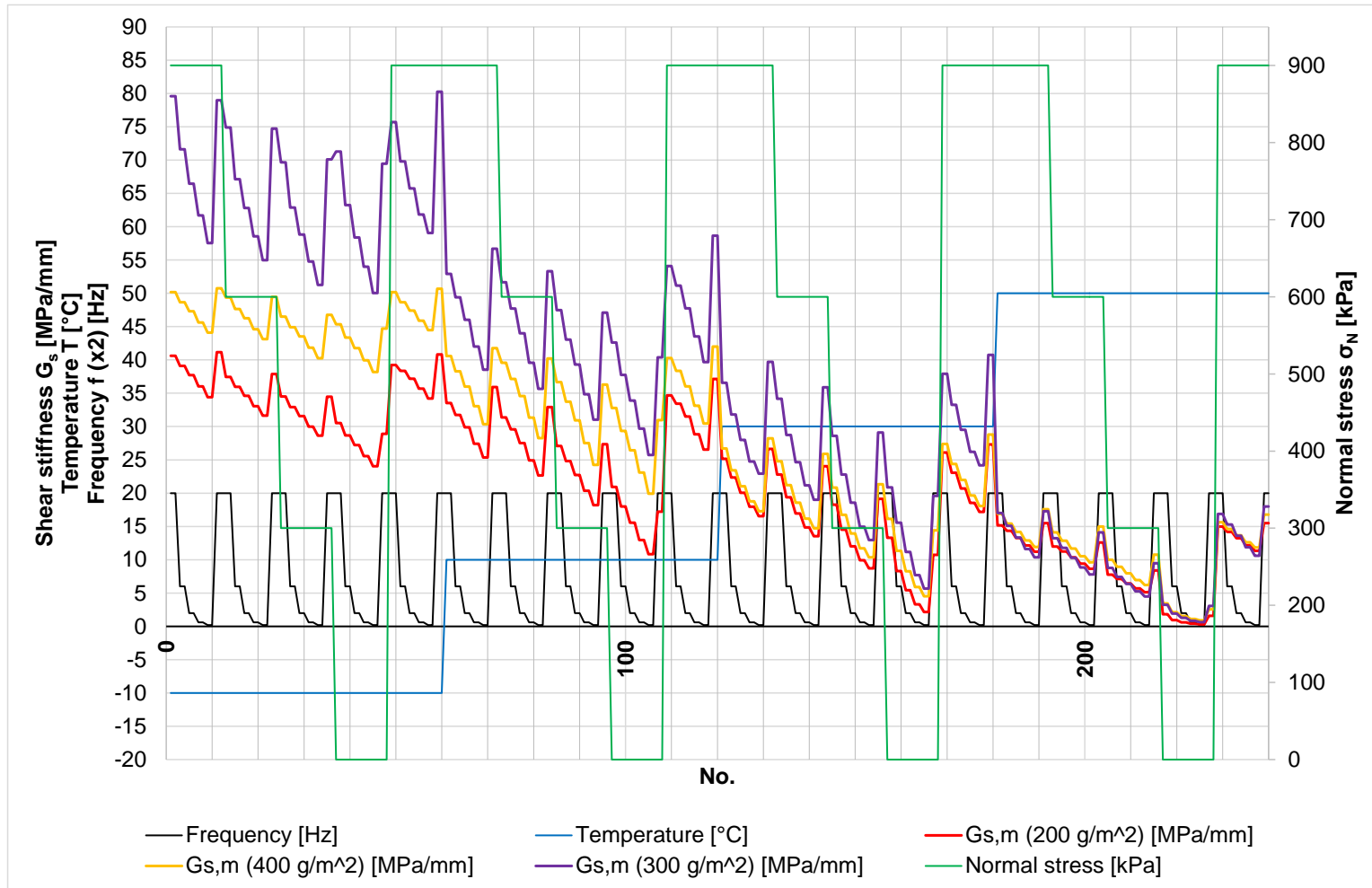


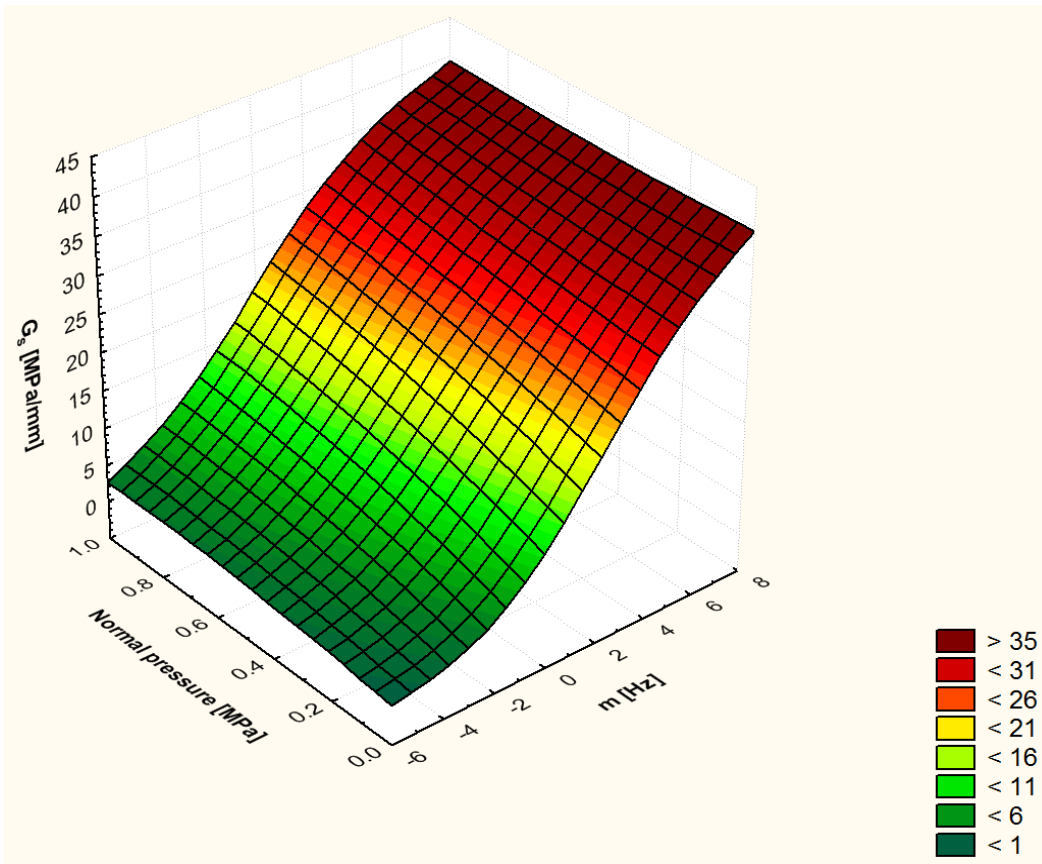


Dynamic shear test









Sigmoidal function

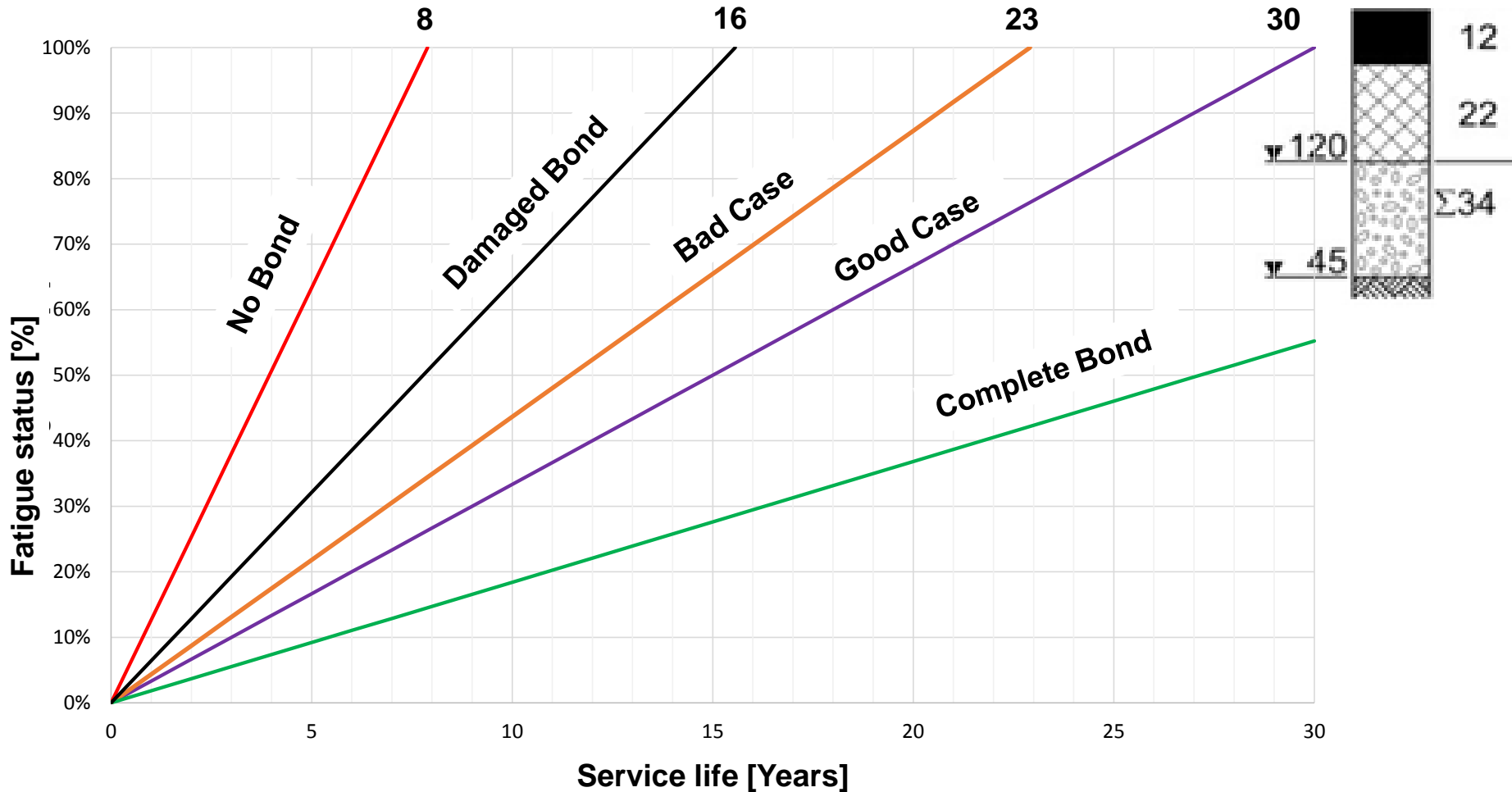
$$G_s = G_{s,min} + \frac{(G_{s,max} - G_{s,min})}{1 + e^{[m \cdot (c \cdot \ln \sigma_N + d) + (j \cdot \sigma_N + k)]}}$$

Example 3 - Layer Bond



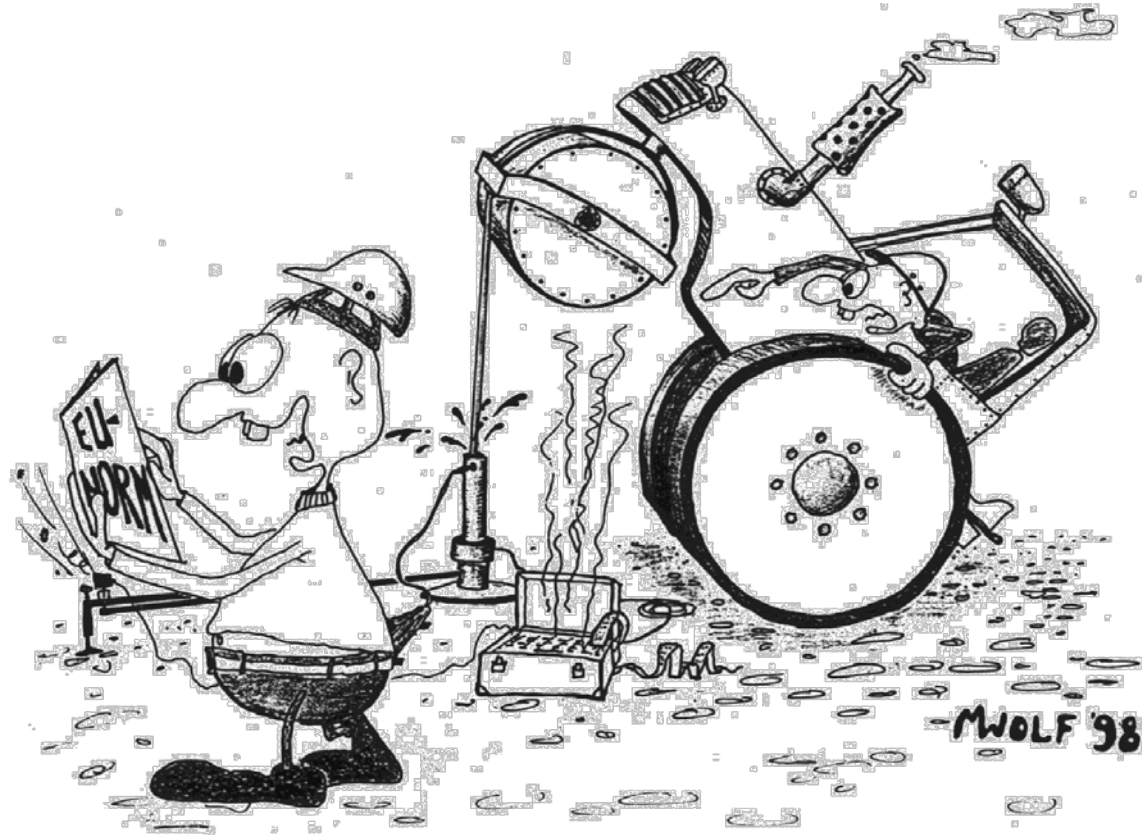
Fakultät Bauingenieurwesen • Institut Stadtbauwesen und Straßenbau • Professur für Straßenbau

Load classification 32...100 Mio 10 t ESALs



Improve the models to describe the material behavior:

- Asphalt: viscos-elastic (complex modulus) + plastic behavior
- Unbound materials: nonlinear-elastic + plastic behavior
- more detailed temperature functions
- Finite element method instead of multi layer theory
- probabilistic instead of deterministic predictions



Thank you for your attention !